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GROUNDWATER IMPACT SCREENING MO
CRANDON PROJECT
WASTE DISPOSAL SYSTEM
PROJECT REPORT 9

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Report on

GROUNDWATER IMPACT SCREENING MO
CRANDON PROJECT
WASTE DISPOSAL SYSTEM
PROJECT REPORT 9

STATE DOCUMENTS
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SEP 17 1984
University of Wisconsin, LRC
Stevens Point, Wisconsin

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March, 1982

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SYMBOL DEFINITIONS

<u>Symbol</u>	-	<u>Definition (Units)</u>
a_L	-	Longitudinal dispersivity (L)
a_T	-	Transverse dispersivity (L)
B_L	-	Longitudinal linear dispersivity coefficient (LL^{-1})
B_T	-	Transverse linear dispersivity coefficient (LL^{-1})
BOT	-	Elevation of bottom of aquifer (L)
C	-	Concentration of solute (MM^{-1})
D	-	Bulk density of the porous media (ML^{-3})
d_C	-	Distance moved by convective flow (L)
D_h	-	Coefficient of hydrodynamic dispersion (L^2T^{-1})
d_L	-	Distance moved by dispersion along flow direction (L)
d_T	-	Distance moved by dispersion normal to flow direction (L)
Δt	-	Length of one timestep (T)
F	-	Head prediction factor (LL^{-1})
I	-	Groundwater gradient in the direction of flow
i	-	node count in x direction
j	-	node count in y direction
h	-	Elevation head of groundwater table (L)
t	-	Time since analysis began (T)
K	-	Hydraulic Conductivity (LT^{-1})
k_d	-	Distribution coefficient (L^3M^{-1})
M	-	Mass of solute injected (L^3)
n	-	Volumetric porosity of the porous media (L^3L^{-3})
Q	-	Inflow to groundwater system (L^3T^{-1})
q	-	Specific discharge (LT^{-1})
r_1, r_2	-	Uniformly distributed random numbers with mean 0 and standard deviation 1.

R_d	-	Retardation coefficient (LL^{-1})
S	-	Storage coefficient (L^3L^{-3})
S_L	-	Standard deviation of the concentration distribution along the flow direction (L)
S_T	-	Standard deviation of the concentration distribution normal to the flow direction (L)
t	-	Time (T)
T	-	Transmissivity - product of K and saturated thickness (L^2T^{-1})
\bar{v}	-	Average pore velocity of groundwater (LT^{-1})
V	-	Darcy velocity of groundwater (LT^{-1})
x	-	Horizontal direction of Cartesian coordinate system
y	-	Vertical direction of Cartesian coordinate system

1.0 INTRODUCTION

This report describes the theory and application procedures of a computer program written by Golder Associates which analyzes the movement of waste facility seepage in an isotropic, saturated, homogenous, porous medium. The program combines a finite difference groundwater flow model with a discrete particle random walk model. Included are the basic theory employed in predicting the transient groundwater gradients and seepage transport, program description, input requirements, output options, a sample problem, and a listing of the program.

This GROUNDWATER IMPACT SCREENING MODEL was developed to provide an easily usable tool for estimating the effect of various approaches to the management of waste products from the proposed Crandon mill on the hydrologic system. The program allows the user to select a proposed waste disposal facility by defining its plan location and a flow history of seepage from each of the system components. The model simulates the dynamic response of the hydrologic system as it is influenced through time by the proposed facility. The results of the analysis consist of a series of "snapshots" of the groundwater table elevation and the seepage concentration in the groundwater over the study area at various times through the simulation. Due to the slow rate of seepage movement in the groundwater system, these "snapshots" are typically provided every 5 to 15 years through simulations that run for 60 to 150 years. In this manner the hydrologic impact can be assessed during the operation of the facility and for many years thereafter.

The program has been written to be simple in terms of its use and ease of modification. Features include:

- Simple input of limited quantity.
- Simple output options of limited quantity to aid remote terminal use.
- Programming is standard FORTRAN IV for maximum compatability between computer systems.
- Programming for maximum internal readability and ease of modifications.
- Relatively simple, unconditionally stable solution algorithms.
- Dimension independent input and output requiring only a consistent dimension set.
- Rapid, relatively inexpensive solution.
- Entire solution matrix is stored in memory to speed execution and minimize program complexity.

Unlike most models of this sort, the code is written to be modified. Different input and output systems will suit different aspects of the project, encouraging tailoring of the program to make the code suit the problem.

The model should be regarded in this application as a screening tool used to evaluate the relative impact on the hydrologic system of various waste disposal systems. The resulting seepage concentration contours do not take into account several factors, as listed below.

- (a) The hydrochemical buffering effect of the unsaturated zone underlying the pond system and in the saturated zone.
- (b) The model is a 2 dimensional representation and does not estimate vertical distribution of concentrations. The concentration contours presented are vertically integrated, i.e., they assume complete mixing of the seepage in the groundwater.

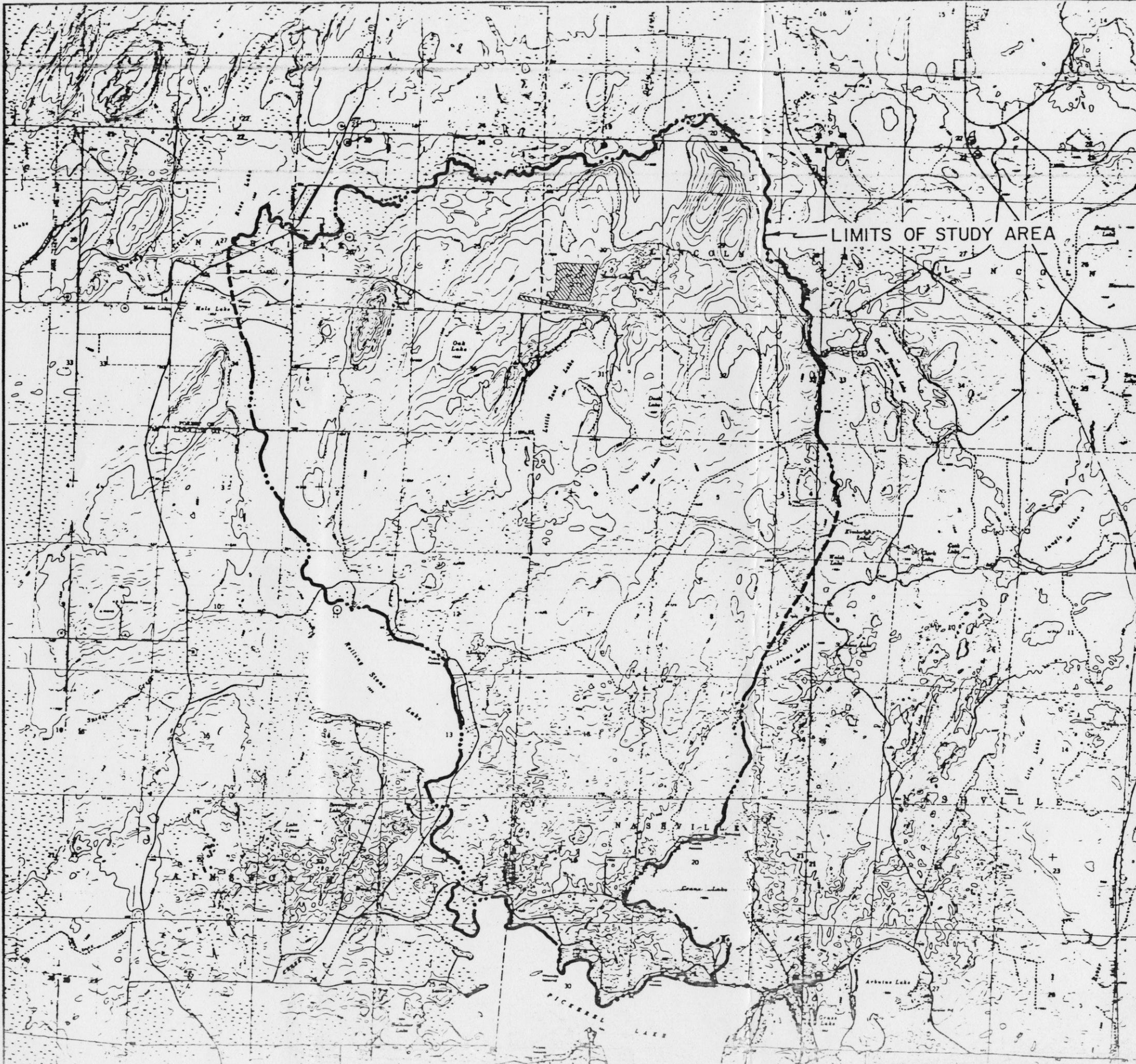
- (c) The effects of seepage fluid dissemination by the process of molecular diffusion.
- (d) Transit time of seepage between leaving the pond bottom and reaching the saturated groundwater zone is not considered.

The model should not be viewed as an exact model of the total Crandon hydrologic system (i.e. hydrochemical and hydrogeological) but rather as a screening tool to aid in determining the optimum site design and location of the proposed Waste Disposal Facility. Future enhancements to the model will allow for consideration of the above factors and perhaps others as the need arises and supporting data becomes available, providing for a groundwater impact assessment tool with a degree of accuracy suitable for its intended use.

2.0 CONCEPTUAL HYDROLOGIC MODEL

Most predictive efforts require the formulation of a model which adequately represents the system to be analyzed. This representation depends heavily upon the expected predictive use of the model. Once the model has been prepared and the user is assured of its accuracy in representing the system in question, predictions can be made by imposing a change in the system and observing the response of the model.

We consider that the model which has been developed for this effort adequately represents the hydrologic system at the Crandon site. The approximate extent of the model is the plan area bounded by Swamp Creek on the north, Ground Hemlock Slough on the east, Pickerel Lake on the south and Rolling Stone and Rice Lakes on the west. The actual boundary limits of the model are the stream channels and lake shores which are recharged by the groundwater. Figure 2.1 shows the study area and assumed boundaries of the hydrogeologic model. In cross-section the model consists of a layer of coarse grained stratified drift material which is effectively continuous over the plan area of the model. An idealized section is shown in Figure 2.2. The saturated thickness of this layer of coarse grained stratified drift varies over the model area. An isopach map of the saturated thickness of the coarse grained stratified drift was constructed from the borehole data, geologic sections and areal trends inferred from the block diagrams (included in Reference 1) and is included in Figure 2.3. The natural water input to the hydrologic model is the rainfall and snowmelt infiltration which percolates into this layer of coarse grained stratified drift. This idealized hydrogeologic system is based on the findings presented in the Geotechnical Investigation⁽¹⁾ and Pump Test and Analysis⁽²⁾ prepared previously by Golder Associates.



NORTH

LIMITS OF STUDY AREA

- LEGEND
- CONSTANT HEAD BOUNDARY
 - - - - NO FLOW BOUNDARY

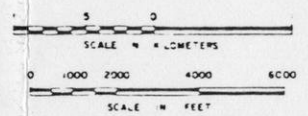


FIGURE 2.1

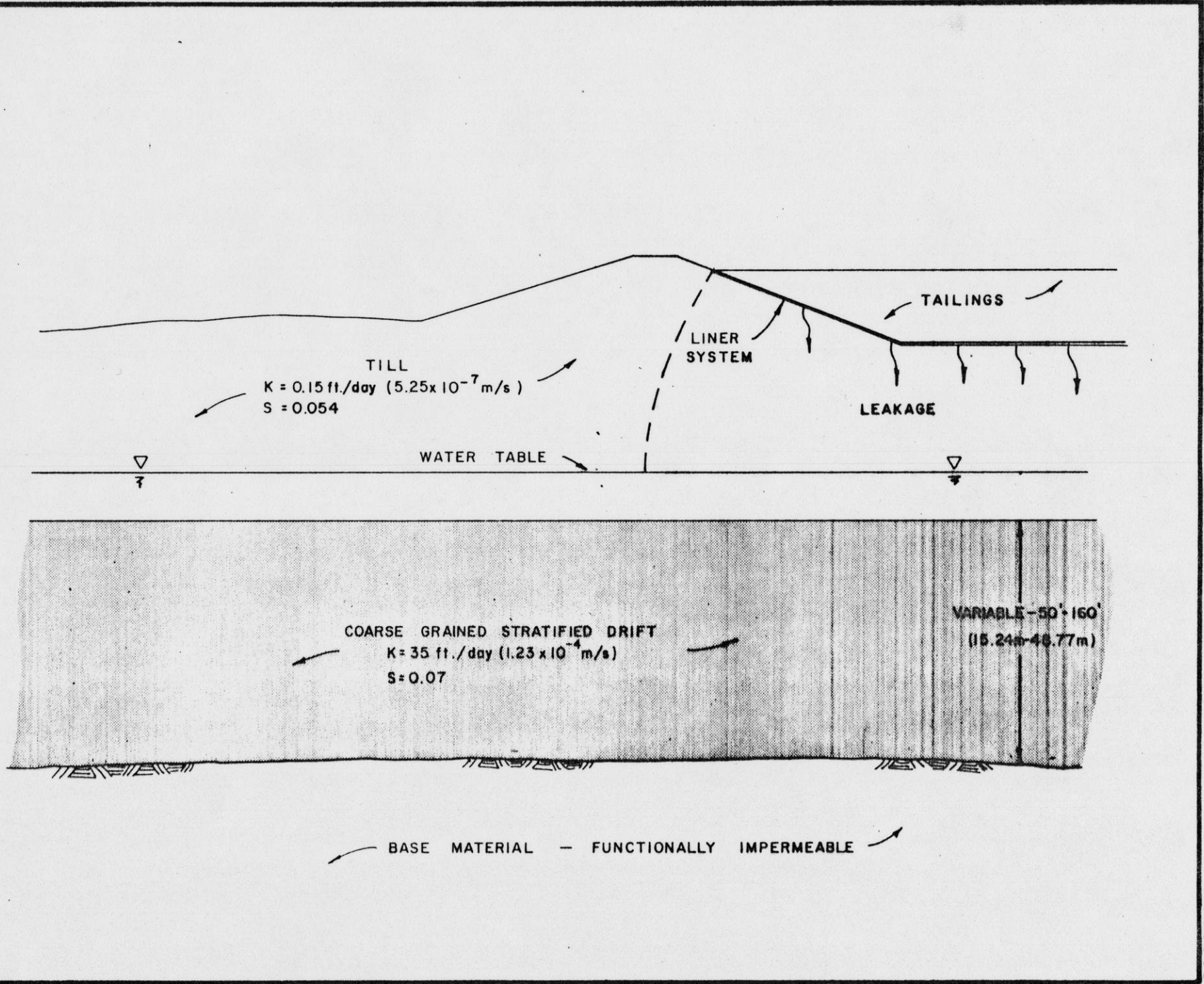
Golder Associates Atlanta, Georgia			
EXXON MINERALS COMPANY CRANDON PROJECT			
TITLE STUDY AREA BOUNDARIES			
SCALE AS SHOWN	STATE WISCONSIN	PROJECT FOREST	
BY SKB	DATE 9-81	APPROVED BY JED	DATE 9/3/81
PROJECT NO. 050-1-80503			

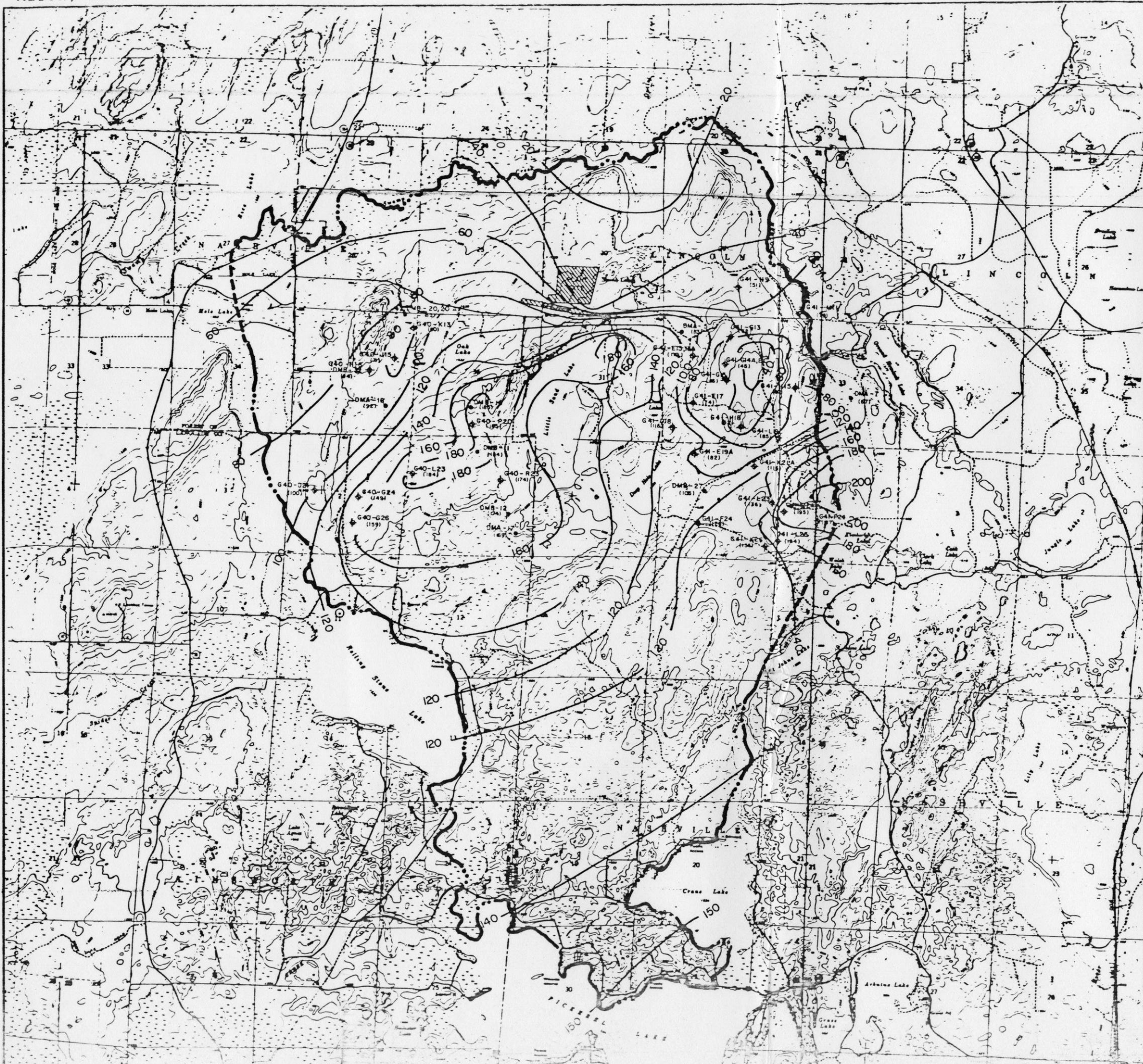
JOB NO. 786085	SCALE NO SCALE
DRAWN CAB	DATE 8-31-81
CHECKED JEB	DWG. NO. 050-1-81112

HYDROGEOLOGIC MODEL
 IDEALIZED CROSS SECTION

EXXON MINERALS COMPANY

FIGURE 2.2





NORTH

- LEGEND**
- · · · — CONSTANT HEAD BOUNDARY
 - — — NO FLOW BOUNDARY
 - 140 — ISOPACH CONTOURS (FEET)
 - G41-L19 (180) GOLDR ASSOCIATES BORING (THICKNESS IN FEET)
 - DMB-27 (105) DAMES & MOORE BORING (THICKNESS IN FEET)

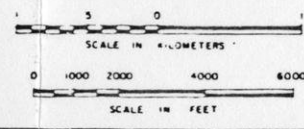


FIGURE 2.3

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Atlanta, Georgia

EXXON MINERALS COMPANY, U.S.A.
CRANDON PROJECT

TITLE
COARSE GRAINED STRATIFIED DRIFT ISOPACH CONTOURS

NO.	DATE	BY	DESCRIPTION
1	11-25-81	CAB	80 CONTOUR @ G41-L19 REVISED

SCALE AS SHOWN	STATE WISCONSIN	COUNTY FOREST
DRAWN BY CAB	DATE 02-13-81	PROJECT 23
CHECKED BY JEB	DATE 2-2-81	
APPROVED BY	DATE	

050-1-80517

The primary use of this model is to predict the concentration distribution of waste facility seepage over the study area through time. The concentrations reported are the percent of seepage in the groundwater assuming complete mixing vertically across the coarse grained stratified drift. The concentration distribution is assumed to be dependent upon the following factors.

- Leakage rate of the waste disposal facility.
- Location and area of the waste disposal facility.
- Groundwater gradients over the study area, i.e. shape of the groundwater table.
- Characteristics of the porous media through which the seepage moves (hydraulic conductivity, porosity, storativity, dispersivity, and retardance).

Diffusion, density gradients and chemical reaction processes are not simulated. The leakage rate, location and area of the waste disposal facility are changes which are applied to the model in a predictive mode. The simulated groundwater gradients and porous media characteristics are input to the model and represent those which are prevalent in the layer of coarse grained stratified drift. The hydraulic conductivity of the coarse grained stratified drift is 2 to 3 orders of magnitude higher than the overlying and underlying layers of till giving rise to seepage transport velocities 100 to 1000 times faster in the coarse grained stratified drift. The hydrologic system is a semi-unconfined system but the model represents it as a single layer water table aquifer for conservative estimates of flow. The coarse grained stratified drift and upper till have hydraulic conductivity values which are different enough to show similar but lagging responses to

stresses such as pumping or recharge. However, the groundwater heads in the two materials equilibrate after the stress has been removed. Since the bottom layer of till has a much lower permeability than the overlying coarse grained stratified drift, it is not affected by waste disposal system leakage which recharges the drift material. It is, therefore, appropriate to formulate the predictive model based on the hydraulic gradient and seepage transport in this layer of coarse grained stratified drift and to use the resulting model in the site screening analyses. Further refinements to the basic algorithms will provide a model which will be appropriate for the final groundwater impact assessment of the waste disposal system.

Once the idealized physical model has been formulated, a method of quantitative cause/effect analysis must be defined. The two basic processes which must be modeled are the changes in the distribution of the elevations of the groundwater table and the distribution of seepage in the groundwater with time. Each process has been represented by a set of equations. Detailed presentations of the theory of these two processes are included as Sections 3 and 4. All mathematical symbols are defined at the beginning of this report.

The methods used to solve the equations which govern the distribution of groundwater heads and seepage concentration are well suited for solution by a computer program. The program developed for this purpose is the GROUNDWATER IMPACT SCREENING MODEL which simulates changes in the groundwater gradients and seepage transport. The program itself is flexible enough to allow variation of the idealized physical model and to simulate a wide range of leakage histories but can represent only the general hydro-

geologic model configuration outlined herein. Therefore, the computer program itself has been termed "the model" or the "GROUNDWATER IMPACT SCREENING MODEL". The combination of the idealized physical model, the mathematical equations, and the computer program represent the model which will be used in a cause/effect predictive mode. In its present form, the cause/effect predictive results are primarily useful in a relative screening mode, but they do represent an upper bound on concentrations to a high degree of confidence. The verification of the program is presented in Section 5; model calibration is included in Section 6; description of the computer program itself is included in Section 7; and Sections 8 and 9 outline the program input requirements and output reports.

3.0 GROUNDWATER FLOW SOLUTION THEORY

The governing equation assumed to represent the transient distribution of groundwater head in a confined, homogeneous, isotropic aquifer in two dimensions is (adapted from Reference 4):

$$\frac{\partial^2 h}{\partial x^2} + \frac{\partial^2 h}{\partial y^2} = \frac{S}{T} \frac{h}{t} + q \quad (3.1)$$

Terms used in this section are defined at the beginning of this report. The above equation cannot be solved directly. By imposing a grid over the problem domain and approximating equation 3.1 in terms of its finite differences between the grid intersection points, a set of simultaneous equations, one representing each grid intersection (node point), can be formulated. The unknown in each equation is the head condition at the node in question at a given time. The known terms are the aquifer properties, T and S, the flow rate q, and the x, y and t values of the solution. This set of simultaneous equations is then solved iteratively using the solution method presented by Prickett and Lonquist⁽³⁾. Figure 8.1 illustrates the Exxon Crandon Project study area overlain by a matrix of discrete node points. This example case will be used throughout this report for demonstration purposes.

Solution of the simultaneous finite difference equations requires definition of the boundary conditions. These boundary conditions are; (1) the initial head configuration at each node point at the beginning of the analysis and (2) either "no-flow" or "constant head" nodes at the periphery of the study area. These boundary conditions are required to reduce the total number of unknowns to that which can be uniquely solved.

Having formulated the system of simultaneous, finite difference equations and defined the boundary conditions, the equations are solved to determine the head at each node. This is done using a modified form of the alternating direction implicit method first presented by Peaceman and Rachford⁽⁵⁾ as described by Prickett and Lonquist⁽³⁾.

"Briefly, the iterative alternating direction implicit method involves first, for a given time increment, reducing a large set of simultaneous equations down to a number of small sets. This is done by solving the node equations by Gauss elimination of an individual column of the model while all terms related to the nodes in adjacent columns are held constant. According to Peaceman, and Rachford (1955), the set of column equations is then implicit in the direction along the column and explicit in the direction orthogonal to the column alignment. The solution of the set of column equations is then a straightforward process.

After all column equations have been processed column by column, attention is focused on solving the node equations again by Gauss elimination of an individual row while all terms related to adjacent rows are held constant. Finally, after all equations have been solved row by row, an 'iteration' has been completed. The above process is repeated a sufficient number of times to achieve convergence, and this completes the calculations for the given time increment. The calculated heads are then used as initial conditions for the next time increment. This total process is repeated for successive time increments. Peaceman and Rachford (1955) point out that this technique is unconditionally stable regardless of the size of the time increment."

The details of the solution methodology are outlined in Prickett and Lonquist⁽³⁾ in "Part 1. Mathematical Background".

The equations formulated thus far apply only to a confined aquifer. The program simulates unconfined and semi-unconfined aquifers by addition of a step to account for the change in transmissivity as the head at a given node rises or falls. The equivalent transmissivity in the x and y directions can be estimated by taking the geometric mean of the transmissivities between the node in question and the next adjacent node, as follows:

$$T_{i,j,x} = K_{i,j} \sqrt{(h_{i,j} - BOT_{i,j}) \cdot (h_{i+1,j} - BOT_{i+1,j})} \quad (3.2)$$

$$T_{i,j,y} = K_{i,j} \sqrt{(h_{i,j} - BOT_{i,j}) \cdot (h_{i,j+1} - BOT_{i,j+1})} \quad (3.3)$$

By correcting the transmissivities at each timestep a reasonable approximation of the behavior of an unconfined aquifer can be obtained. This approximation holds in areas where the phreatic surface is not excessively steep and the assumption of horizontal flow in the aquifer is honored. The predicted drawdowns from a pumping well are too large in the immediate vicinity of the well but are very close to the theoretical drawdowns a short distance from the well, as shown in Section 5.0, Program Verification.

If the timesteps chosen for a simulation are small with respect to the time frame of pumping or injection sequences, the response of the aquifer is fairly consistent through time. That is, if the head at a given node is dropping, then it is a good assumption that it will continue to drop during the next timestep. This assumption can be employed by use of a head prediction step prior to solution of the finite difference equations at each timestep, thereby significantly decreasing the iterations required for convergence. It should be noted that the same

solution will be obtained whether or not the head predictor is employed. The head predictor adjustment is formulated by assuming that the ratio of the change in head between the two prior time cycles (at a given node) is a good estimate of the change in head over the next time cycle. This is shown by the following equations. The heads at times "t-2", "t-1" and "t" have been computed and the heads at time "t+1" are to be solved. A head prediction factor, F, is computed and used as shown below.

$$F = \frac{h_{i,j,t-1} - h_{i,j,t}}{h_{i,j,t-2} - h_{i,j,t-1}} \quad (3.4)$$

$$h_{i,j,t+1} = h_{i,j,t} + (h_{i,j,t-1} - h_{i,j,t}) \cdot F \quad (3.5)$$

Thus, the heads for the next timestep are the adjusted heads as computed above rather than the heads simulated at the end of the preceding timestep. The head prediction factor, F, is constrained between zero and five.

Several other points concerning the solution methodology employed are worthy of note.

Time Step - The finite difference solution of the differential equation of groundwater flow requires that both time and space be discretized. The size of the timesteps employed directly influence the accuracy of the solution obtained. The size of the timestep is dependent upon the size of the node spacing, the aquifer parameters, and the rate at which the aquifer is being pumped or recharged. The timestep should be chosen such that changes in the heads should be gradual through time. However, a timestep which is too small uses excessive computer time and does not gain

significant additional accuracy. Time steps as small as a fraction of an hour are appropriate for a pumping well while monthly or even yearly timesteps are appropriate for gradual infiltration changes. Preliminary hand calculations and sensitivity analyses are recommended to determine the appropriate timestep and grid spacing.

Convergence Criteria - The implicit numerical scheme employed in this program is an approximate solution which is refined through successive iterations. Therefore, some criterion must be defined to determine when the solution has converged to an acceptable accuracy. This convergence criterion is implemented by comparing the difference in heads computed between two successive iterations. When this difference is within an acceptable value, convergence is reached. This program presents the difference in heads between iterations two ways. First, the sum of the change in heads for all nodes is computed. Secondly, the maximum head change at any node is found. Both values are reported and the program uses the sum of head changes as its convergence criteria. The iteration process stops when the sum of the head changes at every node point goes below a user defined limit. This method is most appropriate for regional groundwater simulations. The program also reports the value of the maximum head change at any node and can be easily modified to use this as a convergence criterion. This criterion would be more appropriate when simulating rapid changes to localized portions of an aquifer, such as pumping or injection wells.

Boundary Conditions - Boundary conditions must be defined for solution of the simultaneous finite difference equations. An initial or prior value of the head at each node is defined as either a somewhat arbitrary set of values at the beginning of the simulation or the heads from the immediately preceding timestep. The other set of boundary conditions required imply a known condition at the edge of the study area. These may be either "no-flow" (barrier) nodes or "constant-head" (recharge) nodes. Barrier boundaries are imposed by assigning a very low hydraulic conductivity (10^{-25}) at these nodes. Recharge boundaries are imposed by assigning a very high storage coefficient (such as 10^{25}) at these nodes. It should be realized that the edges of the finite difference grid represent barrier boundaries and effectively constitute a default condition if no boundary is explicitly set.

4.0 SEEPAGE TRANSPORT THEORY

The seepage transport theory employed in the program is based on the standard dispersive flow model summarized in Bear⁽⁶⁾. The governing equation of seepage concentration distribution is:

$$C_{x,t} = \frac{M/n}{(4\pi D_h t)^{1/2}} \exp\left\{\frac{-(x-q/n)t^2}{4D_h t}\right\} \quad (4.1)$$

This equation is similar in form to the equation which defines a normal probability distribution. This allows the following solution methodology.

A known mass of seepage is represented by a single "particle" whose movement in the groundwater system is tracked through time. Hundreds of particles are injected each timestep and their relative density over the study area is mapped at various times through the simulation. Particle density is transformed into seepage concentration by knowing the mass of seepage represented by each particle and the volume of groundwater within each cell. The concentration is the percent pond leakage in the groundwater assuming complete vertical mixing. That is, if the pond leakage had a sulfate concentration of 1500 mg/l and the simulated concentration is 10%, the groundwater would have a sulfate concentration of 150 mg/l. In this formulation, the following assumptions are made (symbols are defined at the beginning of this report):

- Seepage concentration C is low compared to the groundwater density, i.e. the amount of seepage does not affect the groundwater flow.

- The groundwater flow is steady over each timestep and two-dimensional.
- The porous medium is isotropic with respect to dispersion, and thus can be characterized by a longitudinal dispersivity a_L and a transverse dispersivity a_T .
- Any sorption reaction of the seepage with the formation is linear and instantaneous, and can be characterized by a distribution coefficient k_d . The retardation factor (R_d) is defined by:

$$R_d = \left(1 + \frac{D \cdot k_d}{n}\right) \quad (4.2)$$

where D is the bulk density and n the porosity of the porous media.

- seepage is completely mixed in the coarse grained stratified drift layer.

Based on these assumptions, the concentration distribution resulting from injection of a slug of unit mass of seepage into an aquifer at time $t = 0$ at the origin has the following characteristics:

- The centroid of a slug of seepage moves downstream at velocity \bar{v}/R_d , where \bar{v} , the average pore velocity of the fluid, is equal to V/n and V is the Darcy velocity. This distance is denoted as d_c and is defined as:

$$d_c = \frac{\bar{v} \cdot \Delta t}{R_d} \quad (4.3)$$

- The average pore velocity, \bar{v} , is defined as:

$$\bar{v} = \frac{K \cdot I}{n} \quad (4.5)$$

where K is the hydraulic conductivity, I is the gradient in the direction of flow and n is the volumetric porosity.

- With respect to the centroid, the concentration distribution is binormal--the product of a normal distribution in the flow direction with another normal distribution in the transverse direction.
- In the flow direction, the standard deviation of the normal distribution is:

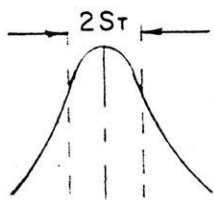
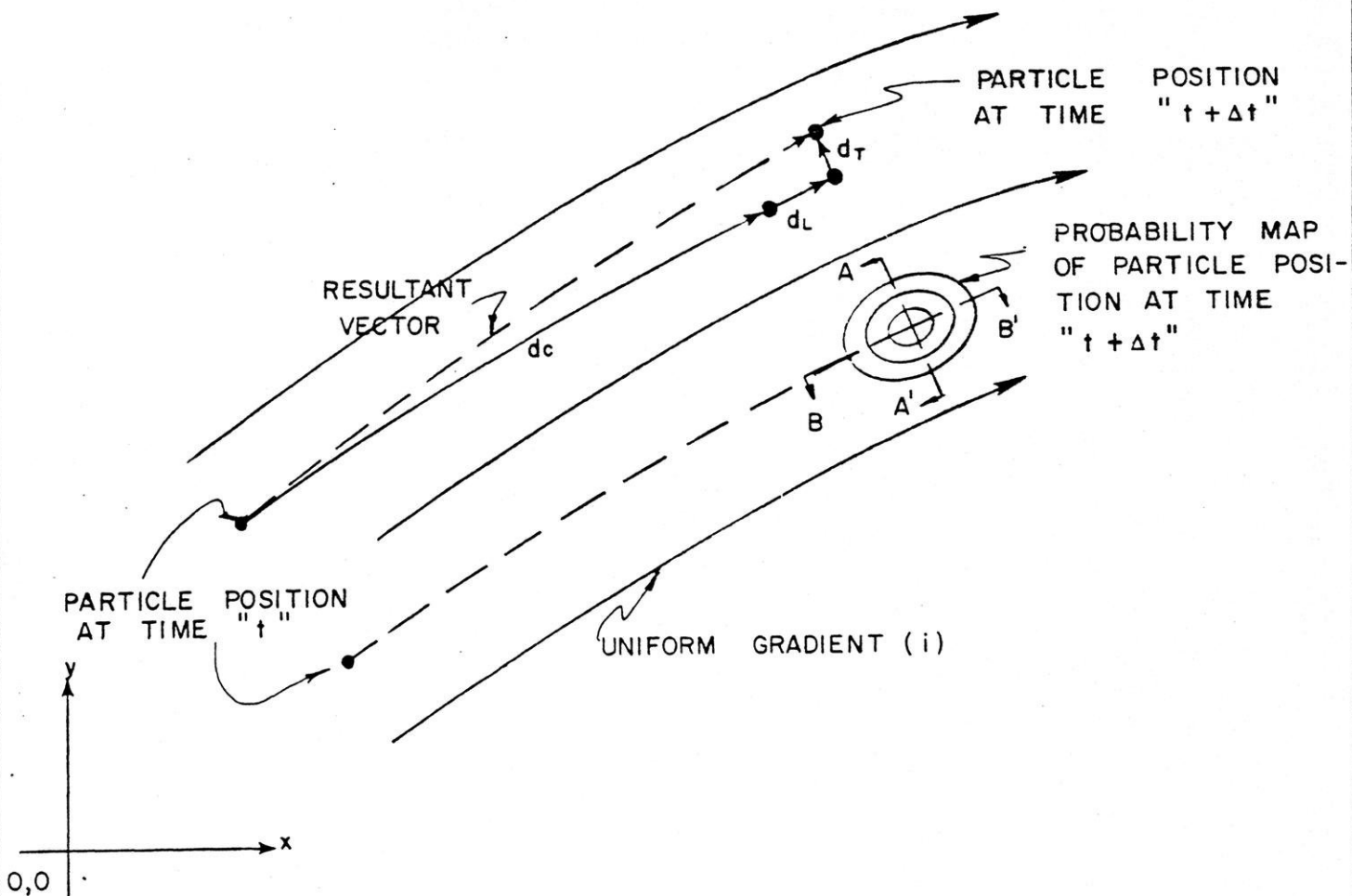
$$S_L = 2a_L \sqrt{(\bar{v}/R_d) \Delta t} = \sqrt{2a_L d_C} \quad (4.6)$$

where d_C is the distance the centroid was moved and Δt is the simulation timestep.

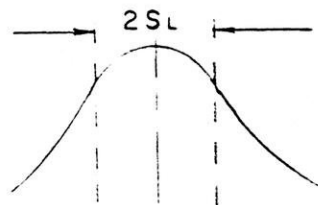
In the transverse direction, the standard deviation is:

$$S_T = \sqrt{2a_T (\bar{v}/R_d) \Delta t} = \sqrt{2a_T d_C} = \sqrt{S_L \frac{a_T}{a_L}} \quad (4.7)$$

The amplitudes of the longitudinal and transverse distributions are $1/S_L$ and $1/S_T$, respectively.



SECTION A-A'



SECTION B-B'

$$d_c = \frac{K \cdot i \cdot \Delta t}{n \cdot R_d} = \frac{V \Delta t}{R_d}$$

$$d_L = r_1 \sqrt{2 \alpha_L d_c} = r_1 s_L$$

$$d_T = r_2 \sqrt{2 \alpha_T d_c} = r_2 s_T$$

JOB NO. 786085	SCALE NO SCALE	CONCEPTUAL PARTICLE MOVEMENT
DRAWN CAB	DATE 5-7-81	
CHECKED JEB	DWG NO 050-1-81111	
Golder Associates		EXXON MINERALS COMPANY
		FIGURE 4.1

Based on this formulation, the seepage transport algorithm used in the program is quite simple. Figure 4.1 illustrates the movement of single particles in a steady state flow field over one timestep. Each particle is moved a deterministic amount (d_c) down gradient from its existing position over a discrete timestep. In addition, each particle is moved a dispersive distance in the longitudinal (d_L) and transverse (d_T) directions. The values of d_L and d_T are computed by selecting a random, normally distributed number (r_1) of mean 0 and standard deviation 1, and multiplying it by the standard deviation of the longitudinal dispersivity:

$$d_L = r_1 \sqrt{2d_c a_L} = r_1 S_L \quad (4.8)$$

Similarly, the transverse dispersion is given by:

$$d_T = r_2 \sqrt{2d_c a_T} = r_2 S_T \quad (4.9)$$

Therefore, the vector describing the movement of a single particle over one timestep is the resultant of the sum of d_c and d_L in the direction of flow and d_T normal to the flow direction. The new position of each particle is computed by decomposing this vector into the X and Y coordinates of the problem domain and adding them to the previous position.

The user should be aware that there are some problems with the application of the basic theory of seepage transport. Experimental data has shown that the values of the dispersivities are very much a function of the scale of the test. Anderson stated in reference 7:

"It is well known that the magnitude of the measured dispersivity changes, depending on the scale at which the measurements are taken. Laboratory experiments designed to measure dispersivity yield values in the range of 10^{-2} to 1 cm, while dispersivities of 10 to 100 meters have been obtained for field problems."

These problems relate to the difficulties in determining an appropriate value of dispersivity. It is recommended that a sensitivity analysis be made to determine the importance of the value of the dispersivity chosen in the problem situation being simulated. This will allow proper perspective to be placed on the selection of dispersivity and will allow the use of a conservative value if appropriate.

5.0 PROGRAM VERIFICATION

As stated at the end of Section 2.0, the "model" is taken to include the conceptual hydrogeologic model, its mathematical formulation and the computer program written to solve the equations. The computer program is further described in Section 7.0. This section presents the verification tests employed to insure that the equations are properly solved by the computer program. This verification process consists of a series of simulations which can be checked by hand calculations.

The two processes modeled by the program are; (1) the change in the groundwater table and (2) the transport of seepage in the groundwater. The change in groundwater heads through time is simulated in the program by a finite difference algorithm. Verification of this algorithm is provided by simulating simple drawdown and recharge cases which can be checked by hand computations. The drawdown case is an infinite, homogeneous, isotropic, unconfined aquifer with a saturated thickness of 100 feet ($K=85$ feet/day, $S=0.05$) from which 2000 gpm is being pumped. Figure 5.1 shows the idealized aquifer, the simulated solution and the hand calculated solution using the Boulton method⁽⁸⁾. The simulated solution shows about 2 feet excess drawdown over the hand computed solution 50 feet from the well but the solutions match at 200 ft. and beyond. This difference is due to the discretization of time and space of the finite difference solution. A grid spacing of 50 feet and an initial timestep of 0.001 days (increasing by a factor of 1.25 each cycle) was used in the modeled analysis. Closer grid spacing would improve the match closer to the well. The recharge case is an infinite, homogeneous, isotropic, unconfined aquifer with an initial saturated thickness of 100 feet ($K=0.5$ feet/day, $n=0.07$) with infiltration being induced at

JOB NO. 786085
 DRAWN CAB
 CHECKED JEB

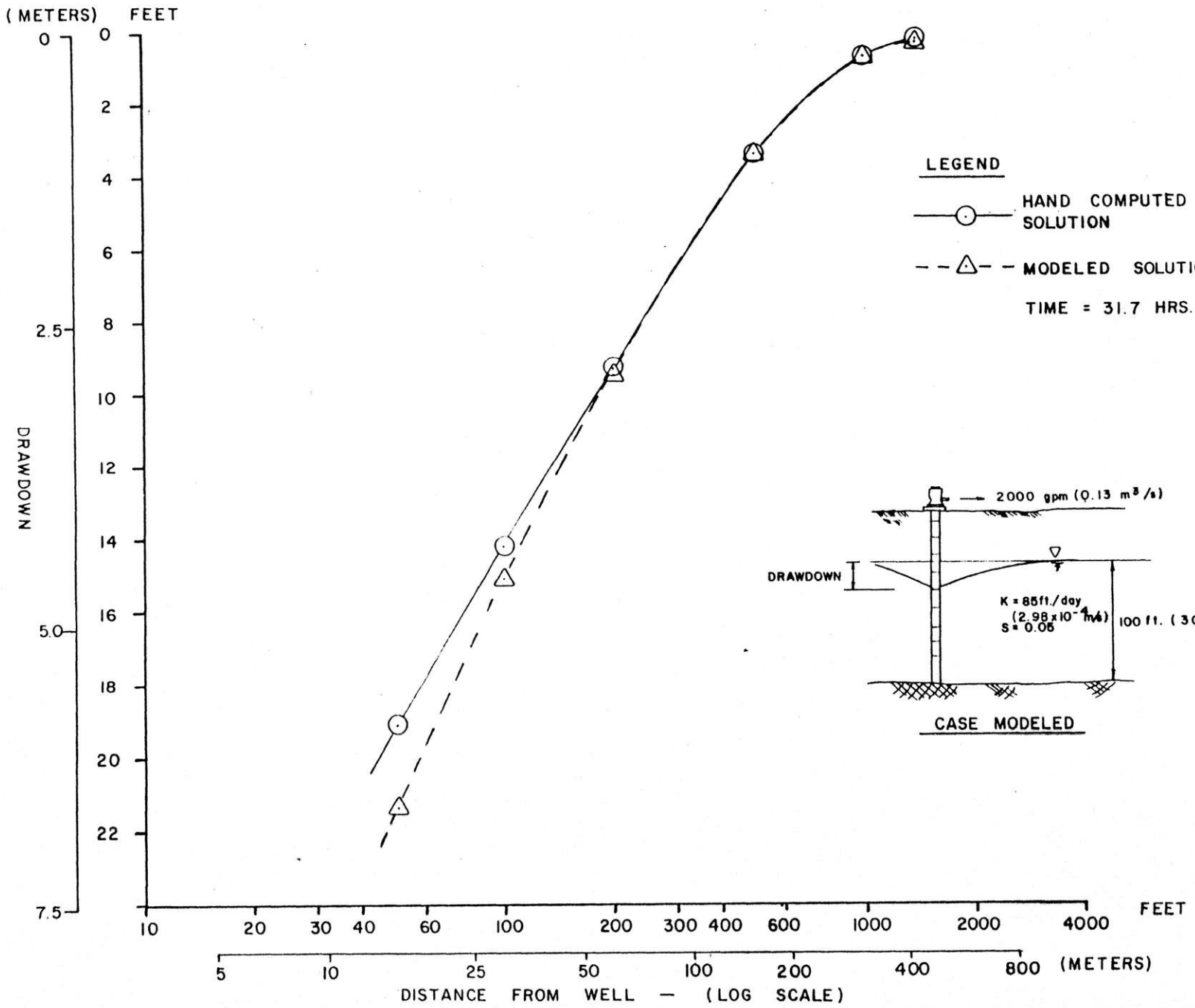
SCALE AS SHOWN
 DATE 7-8-81
 DWG. NO. 050-1-81107

Goldier Associates

GROUNDWATER MODEL VERIFICATION
 PUMPING CONDITION

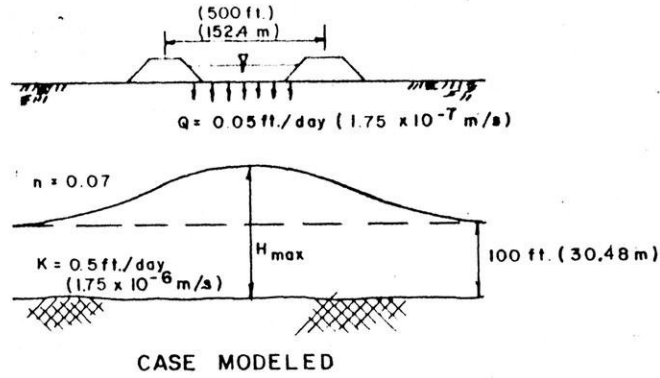
EXXON MINERALS COMPANY

FIGURE 5.1



JOB NO.	786085	SCALE	AS SHOWN
DRAWN	CAB	DATE	7-8-81
CHECKED	JEK	DWG. NO.	050-1-81108

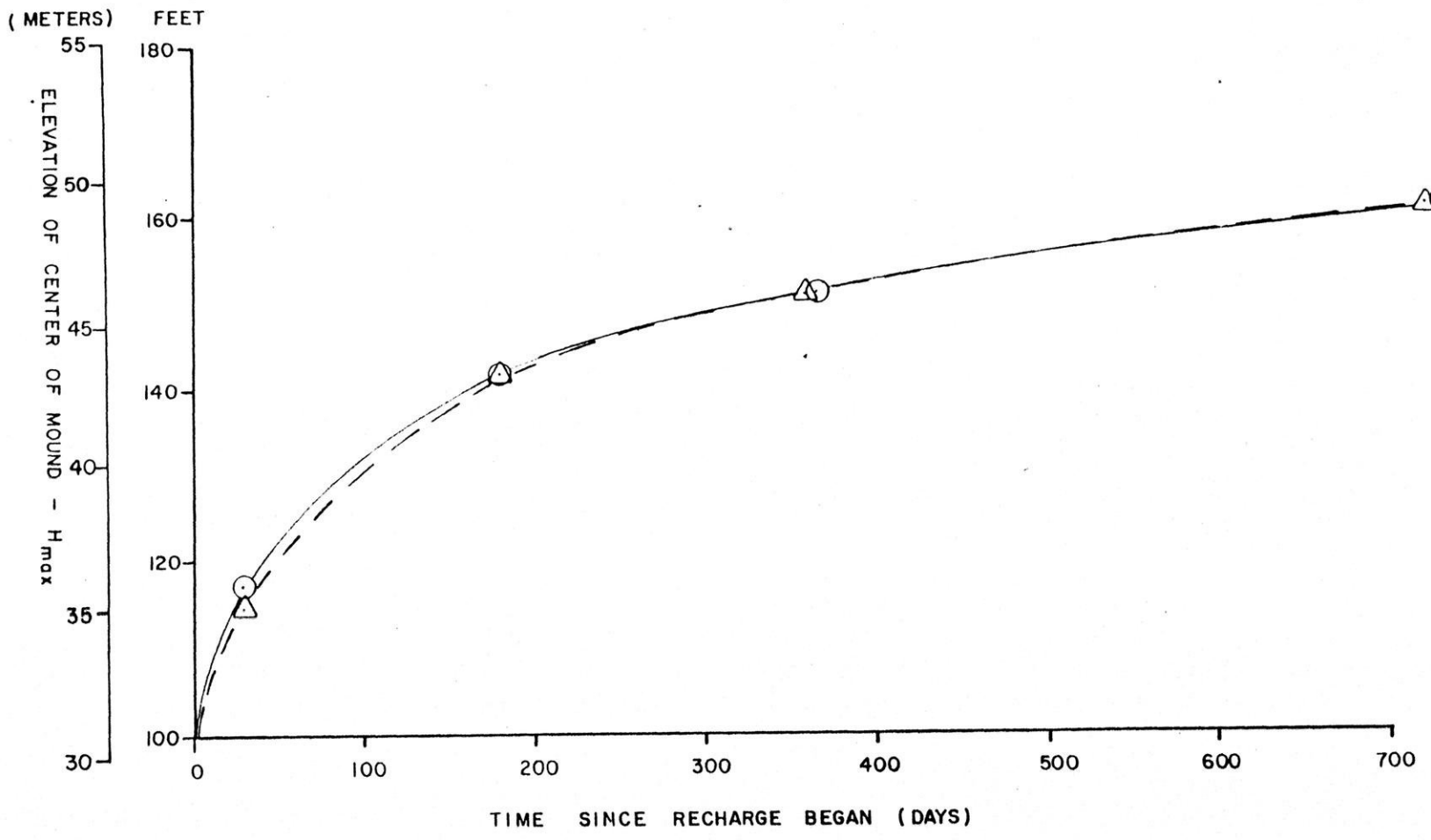
Golder Associates	EXXON MINERALS COMPANY	GROUNDWATER MODEL VERIFICATION RECHARGE CONDITION



LEGEND

○ — HAND COMPUTED SOLUTION

△ — MODELED SOLUTION



a rate of 0.05 foot/day over a 500 foot area. Figure 5.2 shows the idealized aquifer, the simulated solution and the hand calculated solution using the Hantush method⁽⁹⁾. The two solutions show a deviation of about 2 feet during the early part of the simulation. A finite difference grid spacing of 100 feet and a constant timestep of 30 days were used in the modeled analysis.

The second process modeled is the transport of seepage in the aquifer. The verification of this process is provided by modeling an instantaneous slug of a given mass of seepage in a homogeneous, isotropic aquifer ($K = 10$ feet/day, $n = 0.25$) under a uniform, constant gradient of 20 feet/mile. Neither solution considers retardation or chemical change. The hand solution is computed by solving the one dimensional equation of seepage transport. Figure 5.3 shows the aquifer and a cross section of the seepage slug 9 years after injection for the computer simulation and the hand solution.

6.0 MODEL CALIBRATION

Due to the variable nature of the aquifer over the entire study site, some parameters in some areas of the model must be inferred. This is done by modifying the parameters in a logical fashion until the predicted results adequately match those observed in the field. This process is called calibration and is critical to the accuracy of the predictions based on the calibrated model. If too many parameters are inferred or set through calibration an unrealistic parameter set could be selected. This could result in a model calibration which matches the predicted and the observed results adequately but behaves erroneously when changes are imposed. Therefore, measured field data should be used as extensively as possible and unsupported calibration refinements should be avoided.

The head simulation and the seepage transport algorithms may require calibration before using the model. The head solution is calibrated by determining as many of the aquifer parameters as possible through pumping tests, exploration borings, regional water balances, and other applicable sources. The remaining parameters are set to their most likely value. The model can then be calibrated by modifying parameters until the predicted head levels match observed heads in a steady state mode. The final head configuration is usually stored on a RESTART file to use as the initial heads in subsequent simulations. The transient behavior of the model should be calibrated if adequate observed data is available. Transient phenomenon such as spring snow melt or pumping wells are commonly used. If transient observed data is not available the model should still be checked with a reasonable estimate of seasonal variation of infiltration to insure that no extreme fluctuations take place in the model.

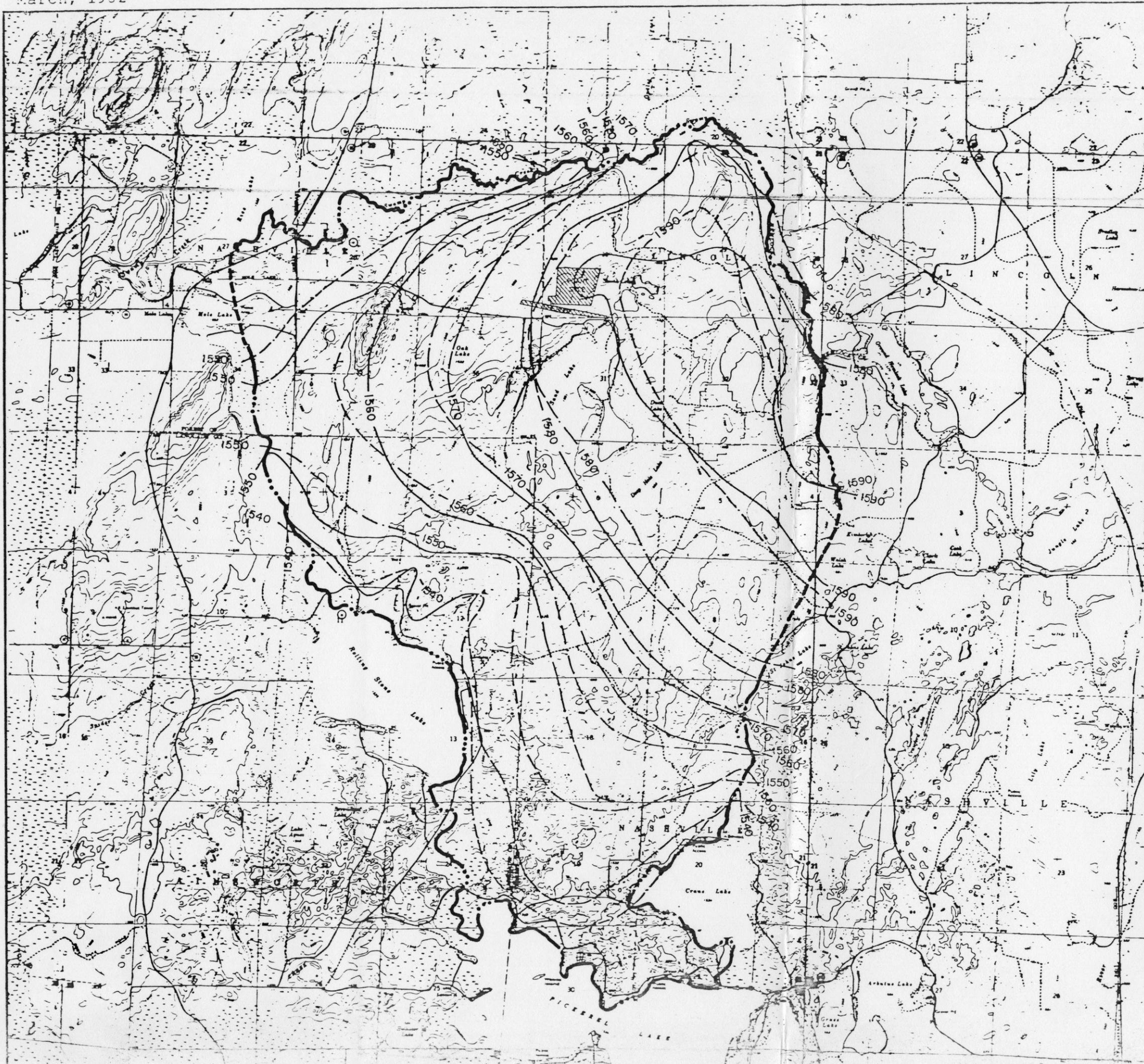
The seepage transport algorithm contains dispersion and retardation parameters which may be calibrated by comparing predicted seepage transport to observed. This is complicated, however, by the scale dependency of the coefficient of dispersivity. Also, such observed data is seldom available requiring the use of values from the literature.

In cases where calibration is nonexistent or weak, sensitivity studies should be made to determine the impact of the parameter values selected on the model results. Quite often some parameters are found to have very little effect over the range of required results and typical values may be used. Also, inferred parameters should be chosen in such a manner to insure conservative results.

The model calibration for the Crandon site was based on a detailed geologic investigation⁽¹⁾, pump test performed in June, 1980⁽²⁾ and Dames and Moores environmental baseline study⁽¹⁰⁾. This site specific data provided the initial set of data from which the calibration effort began. Simulated contours were compared to observed contours to determine the accuracy of the calibration. The observed groundwater contours used are those presented in reference 11 which reflects all data in the area gathered to date and a comprehensive set of piezometer readings in September, 1980.






Several parameters were based upon the above listed sources of information and were not varied during the calibration process. These were the average horizontal hydraulic conductivity of the coarse grained stratified drift of 35 feet/day and storage coefficients of the coarse grained

stratified drift and upper till of 0.070 and 0.054, respectively. To calculate the groundwater head at a given node the model requires a value of aquifer transmissivity which is the product of hydraulic conductivity and saturated aquifer thickness at that point. Values of aquifer thickness were defined for each groundwater head node location. Calibration was achieved by varying the annual net infiltration between the values indicated in the environmental baseline report. A net annual groundwater recharge value of 12.5 inches generated simulated groundwater contours of the same general elevations as those observed. The observed groundwater contours (as presented in reference 11) and simulated groundwater contours are shown on Figure 6.1. The input parameter values are listed in the sample model output in Appendix B.



NORTH

LEGEND

-  CONSTANT HEAD BOUNDARY
-  NO FLOW BOUNDARY
-  SIMULATED GROUNDWATER HEAD CONTOURS
-  GROUNDWATER HEAD CONTOURS FROM FIELD DATA
-  GROUNDWATER HEAD CONTOURS INFERRED FROM FIELD DATA

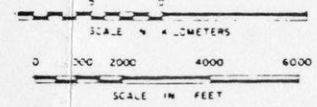


FIGURE 6.1

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Atlanta, Georgia

EXXON MINERALS COMPANY
CRANDON PROJECT

TITLE
OBSERVED AND SIMULATED
GROUNDWATER HEAD CONTOURS

SCALE AS SHOWN	WISCONSIN	FOREST
SKB	3-1-BI	JEB
		5/4/81

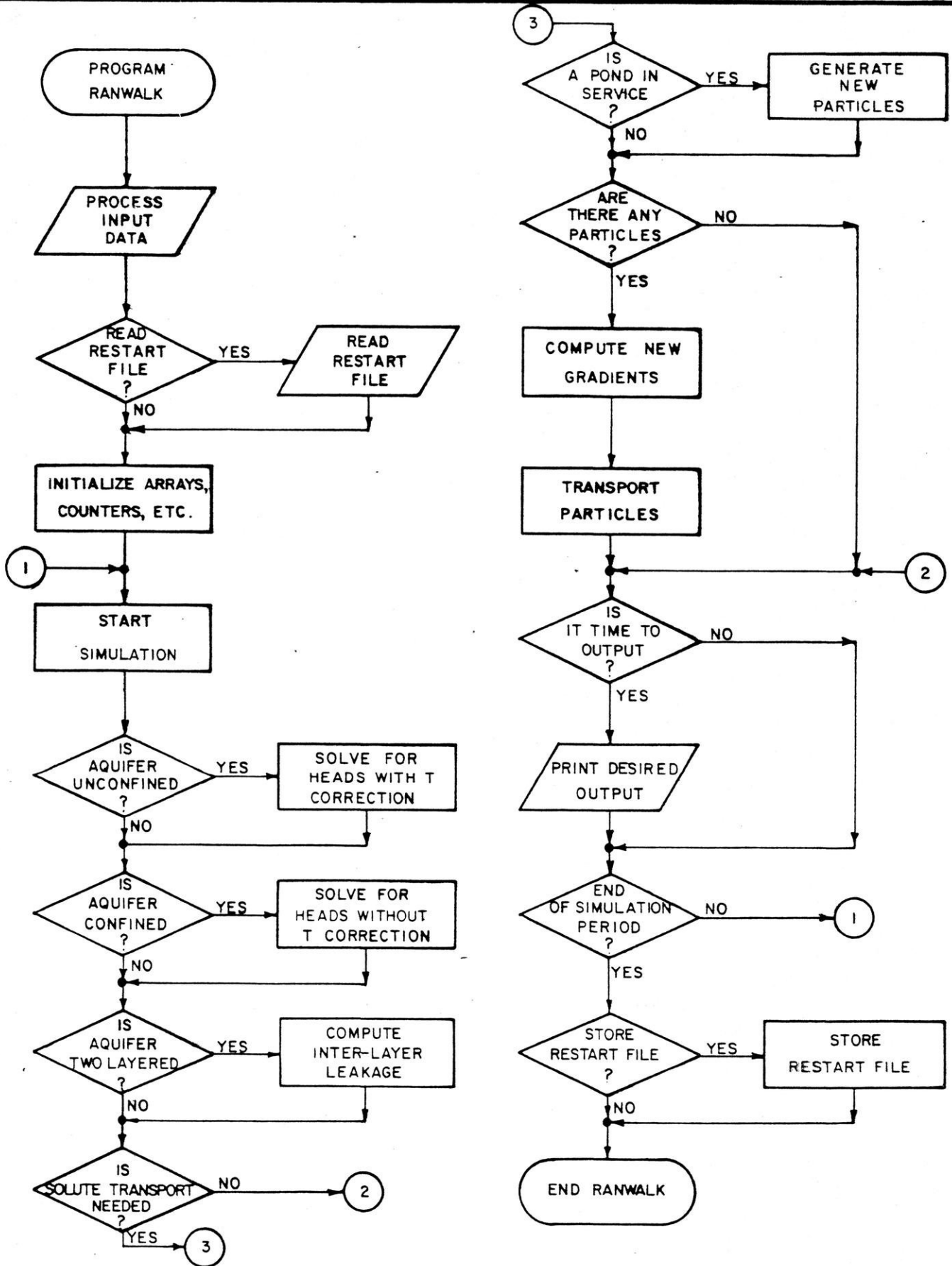
050-1-80504

7.0 PROGRAM DESCRIPTION

The GROUNDWATER IMPACT SCREENING MODEL was developed as a group of service routines which perform specific tasks controlled by a main program. The needed arrays are passed between program units primarily through named COMMON blocks. Flow of logic through the program is kept as linear as practical with entrance and exit from a program unit from the top and bottom only. This practice reduces the complexity of logic flow and minimizes debugging and modification efforts. The program is coded in FORTRAN with as little use of extensions as practical. This allows compatibility of the program with many other computer systems. The program is thoroughly commented and each subprogram contains a description of its function and definition of the major variables used.

The flow chart of the program is shown in Figure 7.1. A brief description of the major subroutines is as follows:

- | | | |
|---------|---|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| RANWALK | - | This is the main program which controls the simulation and calls the major subroutines. |
| INPUT | - | Reads the input data which defines the problem to be simulated, sets various control parameters and loads operational arrays. An echo of the data is provided. |
| IADI | - | Solves the groundwater head at each node by using the iterative alternating direction implicit method as described in Section 3.0. Optionally makes an adjustment for transmissivity change in unconfined aquifers. |
| LEAK | - | Computes leakage between two aquifers based on the hydraulic conductivity of the upper layer and the vertical gradient (not used in present version of the program). |



GAF DRAFTING MEDIA

JOB NO.	786085	SCALE	NOT TO SCALE
DRAWN	SKB	DATE	8-21-81
CHECKED	JEB	DWG NO.	050-1-81110

PROGRAM FLOWCHART

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FIGURE 7.1

- GRADNT - Computes the groundwater gradient in the X and Y directions at each node point and resolves the resultant vector.
- GENERAT - Generates discrete particles based on the particle volume and leakage history input to the model.
- FIND - Finds the cell in which a given x, y point is located.
- MOVE - Transports particles according to the algorithm discussed in Section 4.0.
- ELIM - Eliminates particles which have moved to the boundary areas (streams, lakes, etc.) after each printstep.
- HEDIN - Reads in the restart file, compares critical parameters and echos data to the output report.
- HEDOUT - Stores simulation parameters, head configuration and optionally the particle locations on a restart file.
- INTERP - Interpolates a leakage rate from the leakage history curve. Leakage rate is taken at time $t+0.5(\Delta t)$.
- PLOT - Outputs the particle count matrix at each node, row by row.
- PLOTR - Outputs the head elevation at each node, row by row.
- PLOT3 - Plots a symbolic representation of the distribution of active and inactive particles by assigning a character which corresponds to a range of the number of particles in the cell.
- OUTPUT - Prints the head matrix in block form, not to scale. The origin (node point 1,1) is located in lower left corner; the matrix is partitioned into slices 25 columns wide.
- OUT2K - Prints the head or concentration matrix in block form to a scale of 1" = 2000'. Origin (node point 1,1) is located in the

lower left corner; matrix is partitioned into slices 20 columns wide. In present version reported heads are actual head minus 1500 feet. Concentrations are computed as a percent of leachate in the groundwater contained in each cell.

- OUT3K - Prints symbolic distribution of seepage concentration as a percent groundwater at a 1" to 2000' scale when a seepage grid spacing of DXPAR = 200 ft. and DYPAR = 333.3 ft.
- ERPROC - Prints error messages on output report.

The program has been written so as to minimize the computational effort involved in the innermost loop elements, especially MOVE. Needed constants are evaluated and stored in matrices before entering this loop, and the algebraic expressions have been derived so as to minimize the use of trigonometric and other complex mathematical functions.

The program has several capabilities designed to increase the utility and efficiency of its use.

- Comprehensive set of error codes handled by a separate subroutine.
- Data array dump optionally provided upon trapping of a fatal program error (but not fatal system errors).
- Storage and retrieval of RESTART files which allow simulations to be stopped and restarted with or without change of parameters.
- Several output types, all user selectable.

A listing of the program is included as Appendix C of this report.

8.0 INPUT DATA

8.1 General

As in most simulation programs, the assembly of the input data constitutes the major portion of the effort to apply the model. Data input is mainly keyword directed to allow flexibility in input sequence and facilitate data modification. A description of needed data follows, in Sections 8.2 through 8.7 along with discussion of units. A summary of the input data requirements and input format is included in Section 8.8. The input is explained by use of the Crandon case as an example in which a sample case of a 85 acre pond leaking at 1.0 gpm for 30 years, increasing to 5 gpm thereafter is simulated. The example input is included in Appendix A and the corresponding output is provided as Appendix B.

8.2 Unit Convention

The program requires a consistent set of units. Length and time unit names are input to the model for information purposes only, no unit conversion or checking is performed. Although any consistent set of units can be used computationally, the output portion of the program presently reports in feet and days. All input parameters and output values used in the example case are in these units.

8.3 Definition of the Problem Space Domain

The space domain for the problem is a two-dimensional plane in space. This plane is single valued (i.e., does not double back on itself) but need not be flat or horizontal. All further discussion about the solution space assumes that the plane is reasonably flat, however, as it is assumed that distances in the plane are equal to horizontal

distances. All geometry of the plane is projected onto a horizontal surface.

The solution domain is represented by two grids. The intersection of the grid lines in each grid define a "node point", which is at the center of a "cell". One grid controls the solution of the groundwater head and the other the concentration of seepage. The seepage grid must be an even integer subgrid (or equal to) the head grid. The groundwater head is computed at each head grid node point and the gradients in the x and y direction are computed by interpolating linearly between head node points. The seepage concentration distribution is based on the total seepage mass located in each seepage grid cell, yielding vertically integrated, totally mixed concentrations. The extent of the grids should be selected so as to minimize edge effects. Extending the grids to groundwater interacting rivers and lakes or groundwater ridges is recommended. Other considerations of grid selection are listed below.

- All cells in each grid have the same dimensions, but the two grids may differ.
- The origin of each grid is the origin of the models coordinate system (user may need to correct coordinates to conform to this criteria), and the two grids must use the same origin.
- Points in the domain, including node points, are referred to by their X, Y coordinates. Cells are referred to by their X, Y numbers counting from the origin (1,1).
- All coordinates and cell numbers in each grid are positive.

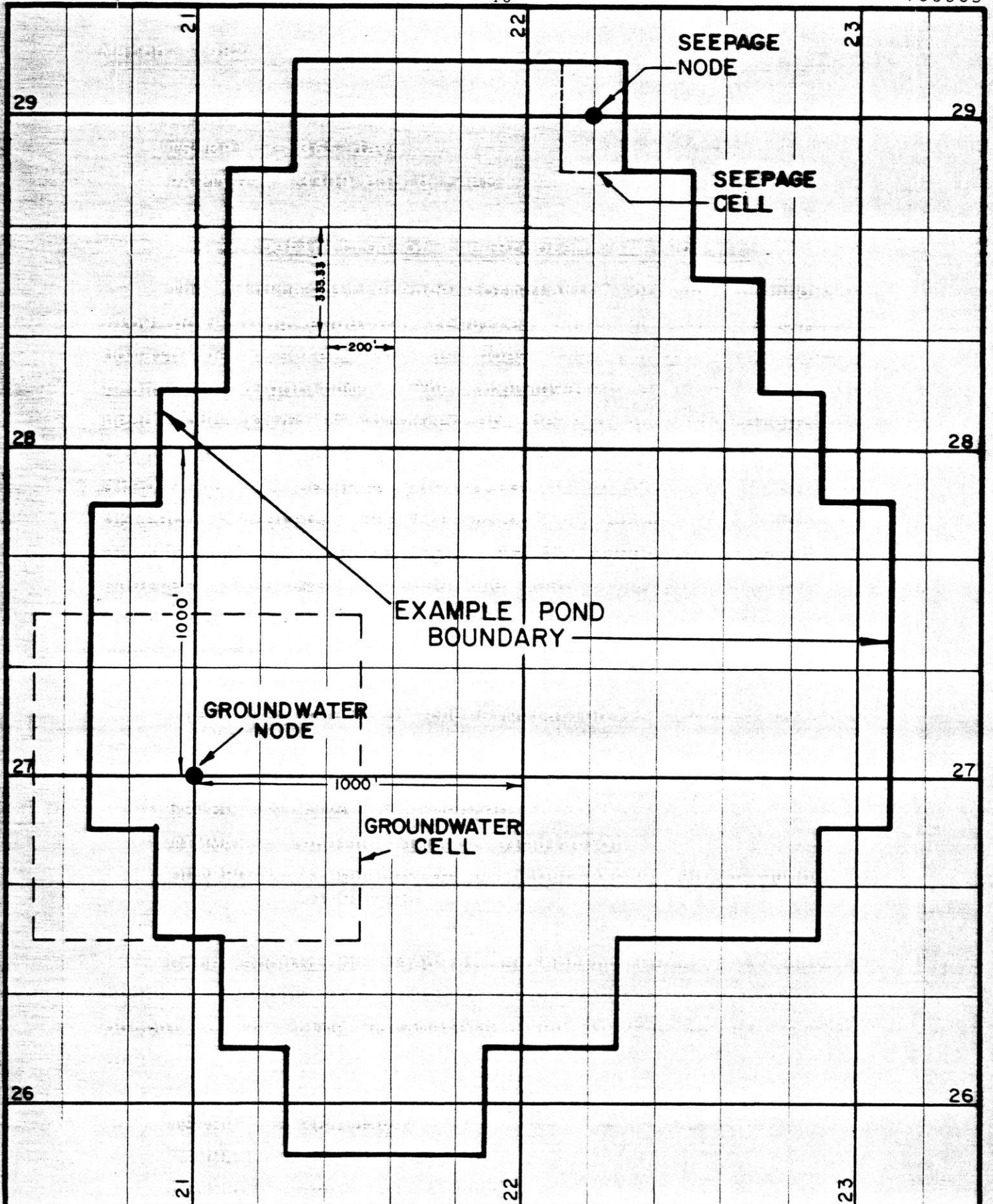
The program has the capability to print the head elevation and seepage concentration matrices at a scale of 1"

to 2000' on a line printer. This is useful for overlaying simulated contours on USGS 7.5 minute quadrangle basemaps, but requires a 10.0 ver.:6.0 hor. distortion of the seepage concentration grid.

Selection of grid spacing and cell size is a compromise between the resolution of the results and cost of the simulation. The finite difference head solution increases in cost as the number of nodes increases due to the number of finite equations to be solved. Concentrations are based on the total seepage mass contained within a seepage grid cell and is reported at the node point. Therefore the broader the seepage grid spacing, the coarser the gradient definition and seepage concentration resolution. Simulation cost increases with the number of seepage grid cells since the total number of particles must be increased to keep an acceptable number of discrete particles in each active seepage cell. A final consideration is that the grid spacing be sufficient to outline features such as boundaries and material types.

The Crandon example, which will be used throughout this manual, is shown in Figure 8.1 which also shows the groundwater head grid. Figure 8.2 shows the subdivision of groundwater grid into seepage subgrids. The grids are defined by eight variables:

DX	=	Size of X dimension of each head cell
DY	=	Size of Y dimension of each head cell
NX	=	Number of head cells in X direction
NY	=	Number of head cells in Y direction
DXPAR	=	Size of X dimension of each seepage cell
DYPAR	=	Size of Y dimension of each seepage cell
NXP	=	Number of seepage cells in X direction
NYPAR	=	Number of seepage cells in Y direction



GAF DRAFTING MEDIA

JOB NO. 786085	SCALE 1" = 400'
DRAWN SKB	DATE 12-7-81
CHECKED JEB	DWG. NO. 050-1-80561

GROUNDWATER AND SEEPAGE GRIDS

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EXXON MINERALS COMPANY

FIGURE 8.2

Two other parameters which effect the resolution and cost of simulation are the timestep and volume of leakage per discrete seepage particle (which controls total particle count). Selection of timestep should consider (1) required output frequency, (2) ratio of gradient induced (advective) particle movement to dispersive movement during each timestep and (3) convergence time for the head solution. The transport components (advective and dispersive) should be compared to prevent overpowering effects of either process. As shown by equations 4.3, 4.6 and 4.7 a short timestep and a large dispersivity or the inverse can result in odd transport behavior in extreme cases. Both the leakage rate and the timestep affect the iterations required for head solution convergence. Further discussion of the timestep follows in the next section.

The volume of seepage to be represented by each particle defines the average number of particles in each cell for a given cell size and leakage history. A maximum of 20,000 particles is allowed by the program. A rule of thumb for particle density is that the user should aim for an average of between 10 and 20 particles seepage per seepage cell within the seepage plume.

The master grid, timestep and particle volume for the sample problem is defined by the following parameters:

DX	=	1000 feet
DY	=	1000 feet
NX	=	28 cells
NY	=	41 cells
DXPAR	=	200 feet
DYPAR	=	333.3 feet
NXP	=	140

NYPAR = 84
TSTEP = 365 days
PARTVL = 3300 cubic feet

8.4 Definition of the Problem Time Domain

The finite difference formulation of the groundwater flow governing equation requires that the simulation time domain be discretized, as does the random walk seepage transport algorithm. The groundwater elevation of each head node point is computed at the end of a timestep, gradients at each particle location are computed between timesteps and all seepage particles injected at the beginning of each timestep. Results output represent the conditions at the end of a timestep. As discussed in Section 8.3, shorter timesteps increase the cost of simulation but are sometimes required for output frequency or head solution convergence.

The time domain of the simulation is defined by three parameters:

TSTEP = size of timestep
TIMMAX = ending time of simulation
OUTFRQ = frequency of printout expressed in number of timesteps

Care should be used in selecting OUTFRQ if several types of output are selected to prevent vast quantities of output. The time parameters used in the sample problem are:

TSTEP = 365 days
TIMMAX = 18,250 days
OUTFRQ = every 10 timesteps

8.5 Definition of Material Types

Several physical characteristics of the aquifer are constant over broad regions of the study area. Input of these characteristics is done by use of material types. Each head cell is given a specific type number which is defined by a given set of parameter values. The program can presently accept up to 10 material types. The parameters which define each material type are:

NVM	=	material type number
K	=	hydraulic conductivity
S	=	storage coefficient
RE	=	normal recharge to aquifer
POR	=	porosity
DL	=	longitudinal dispersivity
DT	=	transverse dispersivity
RT	=	retardation

Several special materials are used for various types of boundaries in the model. A very small (10^{-25}) value for hydraulic conductivity represents a no-flow or barrier boundary; a very high (10^{25}) storage coefficient represents a constant head or recharge boundary; a zero value for porosity (not used in the head situation) represents no seepage transport areas. The barrier and recharge boundaries can be either rows or areas of cells encompassing the project area. "No transport" areas are those areas lying outside of the constant head and no flow boundaries.

The material type distribution over the model area is defined by two input methods. Initially, the entire matrix is set equal to the default material type. Specific head cells can then be set to actual material types by specifying the material type number over the head cell matrix,

leaving a blank or zero for the head cells with the default material type. The material type definitions and their distribution over the head grid is shown in the sample input listed in Appendix A.

8.6 Definition of Leakage Histories

The primary use of the model is to impose a pond leakage at various seepage grid nodes and examine the impact on the head configuration and transport of seepages. The time trace of recharge rates is defined by a set of leakage histories. Up to 20 separate leakage histories can be defined which can overlap in time but not in space. Each leakage history is defined by the following parameters:

NLH	=	Leakage history identifier number
NLPTS	=	number of points on the time/rate leakage curve
NNODES	=	number of seepage grid nodes over which leakage is spread
TM(NLPTS)	=	time ordinates of the leakage curve (NLPTS values must be entered)
RATE(NLPTS)	=	flow rate ordinates of the leakage curve (NSPTS value must be entered)
NODES(NNODES)	=	column numbers (x-direction) of seepage cells (NNODES pairs must be entered)
NODES(NNODES)	=	row numbers (y-direction) of leakage cells (NNODES pairs must be entered)

The flow values in the leakage curve are total flows for the leakage area and are divided evenly over the appropriate seepage cells. Seepage particles are injected randomly over the cells in the leakage area. Leakage values at each simulation timestep are interpolated linearly from

the time/rate curves active at that time. The leakage history used in the sample problem is shown in the sample input listing in Appendix A.

8.7 Definition of Initial Head Configuration

As stated in Section 3, the finite difference solution of the groundwater flow equation requires an initial head configuration, which can be specified at the beginning of each run or by use of a RESTART file, as discussed in the next paragraph. This initial condition can be chosen somewhat arbitrarily except that constant head nodes must be set to their fixed elevation. The solution scheme will iterate to the proper heads at other nodes. This is done through the default head parameter and/or the HEAD matrix input block. The initial groundwater head at nodes defined as constant head nodes must be set to the desired fixed head elevation using the HEAD input block. Heads at all other nodes can be input as zeros or blanks and specified by the default value. The default value should be an estimated areal average of the elevation of the phreatic surface since the further the initial head condition is from the field condition, the more iterations are required for convergence.

The model should be allowed to proceed under steady state conditions through enough timesteps until the change in heads has stabilized prior to imposing the leakage to minimize initial condition effects. Alternatively, the simulation can be terminated when steady state is reached and the heads stored on a RESTART file. Subsequent simulation can then utilize this stored head configuration to define the initial conditions and omit the warm-up period. Heads read from a RESTART file override the default or matrix defined head elevations input to the program.

8.8 Summary of Data Input Requirements

This section defines the format of the input data. Input is in fixed format and must occur in the given order for the first 6 data cards. Subsequent input is keyword directed and data blocks may be arranged in any order.

<u>Card Number</u>	<u>Variable</u>	<u>Format</u>	<u>Description</u>
1	TITLE	8A10	job title
2	TUNIT	A10	time units (information only)
	LUNIT	A10	length units (information only)
3	DX	F10.0	size of X dimension of each groundwater flow cell
	DY	F10.0	size of Y dimension of each groundwater flow cell
	NX	I10	number of groundwater flow cells in X direction
	NY	I10	number of groundwater flow cells in Y direction
	TSTEP	F10.0	timestep size
	TIMMAX	F10.0	ending time for simulation
	PARTVL	F10.0	volume of leakage represented by each discrete particle. Set to zero to inhibit seepage transport simulation.
4	DXPAR	F10.0	size of x dimension of each particle cell
	DYPAR	F10.0	size of y dimension of each particle cell
	NXPART	I10	number of particle cells in x direction
	NYPART	I10	number of particle cells in y direction
	KELIM	I2	indicator to control elimination of particles in no transport areas. 0 = eliminates particles only after each print step (default) 1 = eliminates particles after each time step

5	ISEED	I10	seed for random number generator
	MTYPE	I10	model type 1 = double layer aquifer 2 = unconfined aquifer (make transmissivity correction) 3 = confined aquifer (do not make transmissivity correction)
	MAXIT	I10	maximum number of iterations allowed for each step of finite difference groundwater model
	TOL	F10.0	convergence criteria (sum of differences between actual and calculated heads for all cells)
	OUTFRQ	I10	number of timesteps between report output, first timestep is always printed
	PRINT(10)	10I1	print output selector, each column represents an option which is requested by entering a 1 in the column, as defined below: 1 - dump head, bottom and material type array 2 - dump head array on program generated abort 3 - plots head matrix, 20 columns per page 4 - plots head matrix, 1" to 2000' scale 5 - plots concentration matrix, 1" to 2000' scale 6 - plots symbolic active and boundary particle summary 7 - prints detailed particle count by cell 8 - prints detailed gradients by cell 9 - prints iteration information for finite difference 10- Seepage grid output matrix (at 1" to 2000')

when DXPAR = 200' and
DYPAR = 333.3')

	IHEDIN	I10	file number for file containing restart information
	IHEDOT	I10	file number to output restart information for later use
6	HEAD(1,1)	F10.0	default head elevation
	BOT(1,1)	F10.0	default impermeable bottom elevation
	MATL(1,1)	I10	default material number
	THICK(1,1)	F10.0	default aquifer thickness

The default values of HEAD(1,1) BOT(1,1) MATL(1,1), and THICK (1,1) are automatically assigned to all cells. Modification of default values at various nodes, and initialization of material properties and definition of the leakage histories is done by keyword directed input. Default values are changed by inputting the entire matrix with the overriding value at the proper node and blanks or zeros in locations where defaults are to be used.

Use of keyword input provides increased flexibility in the modification and initialization of parameters and allows any order of data block input. Each keyword references a block of data.

<u>KEYWORD</u>	<u>ACTION</u>
HEAD	modify initial head elevation for any or all head cells
BOTTOM	modify impermeable bottom elevation for any or all head cells
MATERIAL	initialize material parameters for all material types
LEAKAGE	define leakage history parameters

TYPE modify material type designation for any or
 all head cells

 THICKNESS modify aquifer thickness at any or all head
 cells

 END terminate input of data

Block data sets contain the proper keyword as the first card followed by the actual input data values as shown in the sample input in Appendix A.

Block data sets that modify default input values include HEAD, BOTTOM, THICKNESS and TYPE. Information to reference all head cell values in the grid must be included. A zero or blank for any head cell in the block data set results in retention of the default value. The number of head cells per card depends on the particular data set, 8 cells/card for HEAD, THICKNESS and BOTTOM and 80 cells/card for TYPE.

<u>Data Type</u>	<u>Variable</u>	<u>Format</u>	<u>Description</u>
HEAD	HEAD(NX,NY)	8F10.0	(NX*NY)/8+1 cards needed
BOTTOM	BOTT(NX,NY)	8F10.0	(NX*NY)/8+1 cards needed
THICKNESS	THIC(NX,NY)	8F10.0	(NX*NY)/8+1 cards needed
TYPE	MATL(NX,NY)	80A1	one card per row, NY records necessary

Insertion of a blank card results in retention of the default parameter for all cells referenced by that card. In the example input, insertion of the all zero card immediately following the keyword THIC results in retention of the default aquifer thickness for all nodes in grid row 41. This option reduces the amount of data entry necessary to produce a complete data set.

The MATERIAL and LEAKAGE block data sets are used to define the material properties and the pond leakage histories, respectively. Input format is different from that used in the HEAD, BOTTOM, THICKNESS and TYPE data blocks and is explained below.

MATERIAL:

One card must be input for each material type containing the following information (maximum of nine materials can be included). A card with a zero value of NM is required to terminate the material definition sequence.

<u>Variable</u>	<u>Format</u>	<u>Description</u>	<u>Units</u>
NM	I10	material sequence number	-
K	E10.3	hydraulic conductivity	L/T
S	E10.3	storage coefficient	-
RE	E10.3	natural recharge	L^3/T
POR	E10.3	material porosity	-
DL	E10.3	longitudinal dispersivity	L
DT	E10.3	transverse dispersivity	L
RT	E10.3	retardance	L/L

LEAKAGE:

Multiple cards needed to initialize each leakage history. Up to twenty leakage histories can be included. A card with a zero value of NL is required to terminate the LEAKAGE input sequence.

<u>Card No.</u>	<u>Variable</u>	<u>Format</u>	<u>Description</u>	<u>Units</u>
1	NL	I10	sequence number	---
	NLPT	I10	number of points in leakage history	---
	NNODES	I10	number of leaking cells	---
Repeat following card up to 5 times, 4 sets of TM and RATE per card				
2	TM	F10.0	time ordinates of leakage curve	T
	RATE	F10.0	leak rate ordinates of leakage curve	L^3/T

Repeat following card up to 10 times, 10 sets of seepage cell x,y counts per card to define NNODES number of leaking cells

3	IX(1)	I4	cell count in x direction for first leaking cell	---
	IY(1)	I4	cell count in y direction for first leaking cell	---
	IX(2)	I4	cell count in x direction for 2nd leaking cell	---
	IY(2)	I4	cell count in y direction for 2nd leaking cell	---

(Repeat until all leaking cells are specified)

The keyword END, entered left justified, terminates reading of block data sets and initiates the modeling process. Appendix A contains an example input data set for the Crandon site.

9.0 PROGRAM OUTPUT

9.1 Output Reports

The output from the model is divided into 2 parts. The first is an echo of the input which defines the system being simulated. The printout from the example problem used in Section 8 is included in Appendix B. This printout was generated from the input data shown in Appendix A. The end of the first section of output is marked by "INPUT COMPLETE". The second portion of the output is optional and is controlled by the PRINT parameter on the second input card. PRINT allows output options to be selected. PRINT has 10 "flags" which may be set to either 1 or 0 with 1 denoting selection of that particular print option. A typical value for PRINT is shown by column below in which 5 print options are requested.

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)
0	1	0	1	1	1	1	0	0	0

The print options selected by each column are defined below.

1 - Reversed Array Dumps

Prints out head, bottom and material type arrays as a check against input

2 - Array Dump Option

If the program aborts, head values are output at the time of program failure. However, system errors are not trapped and output is not provided when this type of error occurs.

3 - Head Array

Subroutine OUTPUT is called and the head matrix values are printed (not to scale). This option is used during model calibration.

4 - Head Array

Subroutine OUT2K is called and prints the head array during a seepage leakage run to a scale of 1"=2000' for plotting on U.S.G.S. quad sheet base maps. For Crandon simulation heads are printed from a 1500' reference elevation (i.e. reported 90 is actually elevation 1590').

5 - Concentration Array

Subroutine OUT2K prints the seepage concentration (as a percentage of pond leakage in groundwater) at each node for a given printstep at a 1"=2000' scale.

6 - Symbolic Plot

This option calls subroutine PLOT3 to plot the active and boundary particles for the given printstep, in symbolic form. The output matrix prints a single digit for each cell representing the range of particles contained in that cell.

7 - Particle Count

This option prints the abseepage particle count within each cell, acting as a check on the concentration values.

8 - Gradients

A detailed summary of the interpolated groundwater gradient in each cell is printed. Primarily used for initial run checking.

9 - Print Iteration Information

This option prints the iteration number and the two convergence criteria (EPSILON and EMAX) for each iteration. This is useful in detecting non-convergence of the iteration process. A condensed form of this output is provided by default and is sufficient for production runs.

10 - Print Particle Grid Matrix

Prints the symbolic value of the concentration of seepage in each particle grid. With a particle grid spacing of 200 ft. in the x direction and 333 ft. in the y direction this plot comes out at a scale of 1 inch to 2000 ft.

During normal production runs options 2, 4, and 10 are commonly used. Most of the other options are only used to debug failed runs, as they generate a large quantity of output.

By carefully selecting basemap scale and cell dimensions the computer plots can be overlaid on the basemap for direct comparison. A set of diagnostic input data printouts and various data plots are included in Appendix B. Plots prepared from this output are included as Figures 9.1 and 9.2.

Figure 9.1 shows a plot which was generated by overlaying the 1" to 2000' basemap over the printed heads (reported as feet over elevation 1500) and drawing contours. This output allows model calibration to observed data and illustrates changes in the groundwater contours resulting from the construction of the example tailings pond. Figure 9.2 shows the concentration contours over the study area as percent of seepage in the groundwater. These contours were also prepared by overlaying the printout on a 1" to 2000' basemap.

9.2 Interpretation of Output

The primary result of the simulation is the distribution of the concentration of a seepage in the groundwater or surface water system at various times. From the concentration array (select PRINT #10) contours of equal seepage concentration may be drawn at several points in time. These show the spread of the seepage plume at selected times during and after the active life of the tailings ponds system. Alternately, change in concentration through



LEGEND

- CONSTANT HEAD BOUNDARY
- - - NO FLOW BOUNDARY
- CONTOURS SHOWING PERCENT CONCENTRATION OF WASTE LEACHATE IN GROUNDWATER AT TIME = 51 YEARS
- ◻ EXAMPLE POND

NOTE

1. CONCENTRATIONS SHOWN ARE APPROXIMATE.
2. DIFFUSION, RETARDATION AND CHEMICAL ATTENUATION EFFECTS ARE NOT INCLUDED IN ANALYSIS PRESENTED HEREIN.

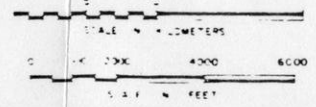


FIGURE 9.2

Golder Associates Atlanta, Georgia		
EXXON MINERALS COMPANY, U.S.A. CRANDON PROJECT		
TITLE SOLUTE CONCENTRATION CONTOURS		
SCALE AS SHOWN	STATE WISCONSIN	TOWNSHIP FOREST, LANGLADE
PROJECT CAB	12-7-81	
DATE		
050-1-80563		

NO.	DATE	BY	DESCRIPTION

time at a certain cell can be computed to yield a time-concentration curve at points of interest.

The seepage concentration in surface waters can be determined from the volume of seepage discharged from the groundwater. This requires knowledge of the flow in the river or lake over the simulation period. A time trace of volume of seepage entering a given reach of stream or lake can be computed by using the detailed particle counts (select PRINT #7). Concentrations are computed by applying the mass of seepage to the appropriate volume of water. Linkage patterns and travel or flushing times of lakes and streams are required to properly account for the movement of seepage through the surface water system and to determine the time-concentration relationship in surface waters.

9.3 Error Codes

The program performs several checks to prevent system errors (division by zero, etc.) and to insure against illogical problem formulation. These errors are flagged in the various subroutines by setting an error code and calling the subroutine ERPROC. Error codes less than 99 are considered nonfatal information errors and do not cause the program to abort. Error codes equal to or greater than 100 are fatal and cause processing to terminate. Array dumps can be requested upon program abort by selecting PRINT #2. A definition of the error codes is listed below.

<u>Error Code Number</u>	<u>Subroutine Referenced</u>	<u>Description</u>	<u>Remedy</u>
101	IADI	Number of iterations needed for convergence exceeds Maximum number of iterations per timestep	- increase tolerance - increase number of iterations - recalibrate model

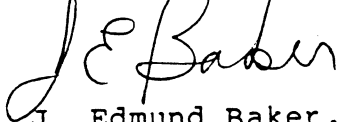
102	INPUT	The end of data cards was encountered before all parameters initialized, as, necessary parameters were never initialized	- make sure data file exists - review input data set for missing cards or data blocks
103	LEAK	The head falls below the bottom of the aquifer (negative head)	- review problem formulation - check pumpage rate
104	INPUT	The data block name used to define a data set (MATERIAL, HEAD, etc.) is not one of the six legal forms	- check input data set for misspelled data set keyword (only first four letters are used)
105	INPUT	Input data does not result in selection of the correct single layer confined model (only used for Crandon simulation)	- make sure variable M=3
106	HEDIN	The data file that is supposed to contain head and particle information does not contain enough information for a restart or does not exist	- check restart data file for completeness - check for correct file number - check for existence of restart file
107	GENERATE	More than 20,000 particles have been generated	- review and redefine particle history
108	HEDIN	Time unit mismatch between simulation run and restart file	- check time units on input data and restart file
109	HEDIN	Length unit mismatch between simulation run and restart file	- check length units on input data and restart file
110	HEDIN	Grid spacing mismatch between simulation run and restart file	- check grid spacing on input data and restart file
111	HEDIN	The restart file is empty	- check JCL string for misspelled data name - check to make sure input data file exists

10.0 SUMMARY

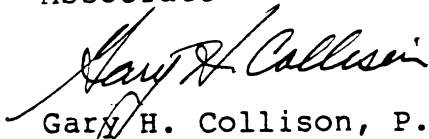
The GROUNDWATER IMPACT SCREENING MODEL presented in this report was developed by Golder Associates to aid in the evaluation of alternative sites and facility layouts for a proposed tailings disposal system at Exxon's Crandon, Wisconsin project. The model uses a finite difference groundwater flow solution and simulates seepage transport using the "random walk" algorithm because of its simplicity and rapid solution. These features are necessary since many analyses are required at the planning stages of site evaluation.

This model is presently being applied at Exxon's Crandon, Wisconsin project for determination of relative impacts of various siting alternatives and is functioning well. In this application, the change in groundwater system gradients are simulated over the analysis period. This allows the relative evaluation of the effects of pond seepage on the groundwater system, including both the effect on the groundwater gradients and the concentration distribution of pond seepage throughout the study area.

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APPENDIX A

Example Input

0	0	83	87	93	98	116	143
164	175	181	181	172	163	155	147
140	133	123	112	98	93	93	115
147	163	0	0				
0	0	87	93	98	114	140	160
182	182	178	176	174	165	159	150
141	133	122	113	110	109	120	134
160	187	205	0				
0	0	0	97	110	135	153	168
175	174	174	174	174	168	163	152
143	135	125	115	115	122	129	138
195	197	200	0				
0	0	0	100	121	142	165	170
171	172	173	174	173	168	163	155
147	135	123	112	120	125	130	140
160	176	0	0				
0	0	0	0	123	142	164	165
166	167	166	165	164	160	158	150
143	134	120	110	125	128	133	137
140	155	0	0				
0	0	0	0	124	139	152	158
160	159	158	157	154	152	150	143
137	127	115	115	125	127	130	133
138	146	0	0				
0	0	0	0	0	135	142	147
151	152	153	152	147	145	143	138
130	122	115	115	123	127	130	135
137	0	0	0				
0	0	0	0	0	0	0	0
142	143	144	143	142	140	136	130
125	117	110	116	125	127	130	133
0	0	0	0				
0	0	0	0	0	0	0	0
0	138	138	136	135	130	126	120
115	110	115	120	125	128	130	133
0	0	0	0				
0	0	0	0	0	0	0	0
0	0	130	127	125	123	120	113
107	113	120	123	125	127	132	0
0	0	0	0				
0	0	0	0	0	0	0	0
0	0	0	120	112	110	105	112
118	121	123	125	127	131	133	0
0	0	0	0				
0	0	0	0	0	0	0	0
0	0	0	110	114	116	120	122
123	125	128	130	132	134	136	0
0	0	0	0				
0	0	0	0	0	0	0	0
0	0	0	120	122	124	126	127
128	129	130	132	134	136	137	0
0	0	0	0				
0	0	0	0	0	0	0	0
0	0	0	123	124	125	126	127
128	129	131	133	135	137	138	0
0	0	0	0				
0	0	0	0	0	0	0	0
0	0	0	125	126	127	128	129
132	133	135	137	138	139	0	0
0	0	0	0				

APPENDIX B
Example Output

GOLDER ASSOCIATES GROUNDWATER COMPUTING SYSTEM

EXXON MINERALS CRANDON PROJECT

GROUNDWATER IMPACT SCREENING MODEL

MODIFICATION 6.1 03 DECEMBER 1981

RUN TIME: 12.57.49. RUN DATE: 81/12/07.

EXXON-CRANDON PROJECT REPORT 9.2 EXAMPLE CASE

--- UNITS ---

TIME UNITS = DAYS LENGTH UNITS = FEET

--- PROBLEM DEFINITION ---

GRID SPACING IN X DIRECTION (FEET)	DX =	1000.00
GRID SPACING IN Y DIRECTION (FEET)	DY =	1000.00
NUMBER OF NODES IN X DIRECTION	NX =	28
NUMBER OF NODES IN Y DIRECTION	NY =	41
TIME STEP (DAYS)	TSTEP =	365.00
MAXIMUM RUN TIME (DAYS)	TIMMAX =	18250.00
PARTICLE VOLUME (CU.FEET)	PARTVL =	3300.0000
SUBGRID SPACING IN X DIRECTION (FEET)	DXPAR =	200.
SUBGRID SPACING IN Y DIRECTION (FEET)	DYPAR =	333.
NUMBER OF NODES IN SUBGRID X DIRECTION	NXPAN =	140
NUMBER OF NODES IN SUBGRID Y DIRECTION	NYPAR =	123
PARTICLES ELIMINATED ON PRINT STEP	KELIM =	0
RANDOM NUMBER	ISEED =	7823976
MODEL TYPE : CONFINED ONE LAYER	MTYPE =	3
MAXIMUM ITERATIONS PER TIME STEP	MAXIT =	100
ITERATION TOLERANCE (FEET)	TOL =	1.
FREQUENCY OF OUTPUT (NO. OF TIMESTEPS)	OUTFRQ =	10
PRINTOUT OPTIONS (SEE DETAILS BELOW)	NDATPR =	0101000011
RESTART CONDITION READ FROM FILE NUMBER	IHEDIN =	10
RESTART CONDITION WRITTEN TO FILE NUMBER	IHEDOT =	11

--- PRINTOUT OPTIONS ---

SELECTED?

DUMP HEAD, BOTTOM AND MATERIAL TYPE ARRAY	NO
DUMP HEAD ARRAY ON PROGRAM GENERATED ABORT	YES
PLOT HEAD MATRIX, 20 COLUMNS PER PAGE	NO
PLOT HEAD MATRIX, 1 INCH TO 2000 FEET SCALE	YES
PLOT CONCENTRATION MATRIX, 1 INCH TO 2000 FEET SCALE	NO
PLOT SYMBOLIC ACTIVE AND HUNGARY PARTICLE SUMMARY	NO
PRINT DETAILED PARTICLE COUNT BY CELL	NO
PRINT DETAILED GRADIENTS BY CELL	NO
PRINT ITERATION INFORMATION FOR FINITE DIFFERENCE	YES
PRINT SYMBOLIC CONCENTRATION CONTOURS (SUBGRID)	YES

--- DEFAULT VALUES ---

GROUNDWATER HEAD ELEVATION	=	1590. FEET
BASE OF AQUIFER ELEVATION	=	1450. FEET
MATERIAL TYPE	=	9
AQUIFER THICKNESS	=	70.000 FEET

--- MATERIAL TYPE MATRIX ---

CELL NUMBER Y DIRECTION	CELL NUMBER IN X DIRECTION															
	1								2							
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
41	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
40	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
39	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
38	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
37	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
36	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
35	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
34	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
33	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
32	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
31	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
30	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
29	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
28	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
27	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
26	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
25	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
24	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
23	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
22	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
21	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
20	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
19	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
18	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
17	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
16	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
15	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
14	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
13	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
12	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
11	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
10	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
8	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
7	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
6	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
5	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
4	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
3	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
2	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9
1	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9

--- MATERIAL TYPE DEFINITION ---

NUMBER	PERMEABILITY (FEET/DAYS)	STORATIVITY	RECHARGE (CU.FEET/DAYS)	POROSITY	LONGITUDINAL DISPERSIVITY (FEET)	TRANSVERSE DISPERSIVITY (FEET)	RETARDATION
1	.37E+02	.70E-01	.29E+04	.250E+00	.200E+03	.200E+02	.100E+01
2	.37E+02	.54E-01	.29E+04	.250E+00	.200E+03	.200E+02	.100E+01
3	.37E+02	.54E-01	.29E+04	.250E+00	.200E+03	.200E+02	.100E+01
4	.37E+02	.54E-01	.29E+04	.250E+00	.200E+03	.200E+02	.100E+01
5	.10E-24	.54E-01	0.	0.	0.	0.	.100E+01
6	.37E+02	.10E+26	.29E+04	0.	0.	0.	.100E+01
7	.37E+02	.70E-01	.29E+04	.250E+00	.200E+03	.200E+02	.100E+01
8	.37E+02	.54E-01	.29E+04	.250E+00	.200E+03	.200E+02	.100E+01
9	.37E+02	.70E-01	.29E+04	0.	0.	0.	.100E+01

--- LEAKAGE HISTORIES ---

HISTORY NO. 1 NO. OF LEAKAGE CURVE POINTS = 5 NO. OF NODES = 85

LEAKAGE CURVE

TIME (DAYS)	RATE (CU.FEET/DAYS)
0.00	192.00
10950.00	192.00
10951.00	962.00
18250.00	962.00
999999.00	962.00

NODAL COORDINATES OF LEAKAGE FROM SOURCE 1

X, Y	X, Y	X, Y	X, Y	X, Y	X, Y	X, Y	X, Y	X, Y	X, Y	X, Y	X, Y	X, Y
108, 78	109, 78	110, 78	107, 79	108, 79	109, 79	110, 79	111, 79	112, 79	106, 80	107, 80	108, 80	109, 80
109, 80	110, 80	111, 80	112, 80	113, 80	114, 80	115, 80	105, 81	106, 81	107, 81	108, 81	109, 81	110, 81
110, 81	111, 81	112, 81	113, 81	114, 81	115, 81	116, 81	105, 82	106, 82	107, 82	108, 82	109, 82	110, 82
110, 82	111, 82	112, 82	113, 82	114, 82	115, 82	116, 82	105, 83	106, 83	107, 83	108, 83	109, 83	110, 83
110, 83	111, 83	112, 83	113, 83	114, 83	115, 83	116, 83	106, 84	107, 84	108, 84	109, 84	110, 84	111, 84
111, 84	112, 84	113, 84	114, 84	115, 84	107, 85	108, 85	109, 85	110, 85	111, 85	112, 85	113, 85	114, 85
114, 85	107, 86	108, 86	109, 86	110, 86	111, 86	112, 86	113, 86	108, 87	109, 87	110, 87	111, 87	112, 87

**** INPUT COMPLETE ****

---AQUIFER THICKNESS (FEET)---

Y= 41																	
1)	70.00	2)	70.00	3)	70.00	4)	70.00	5)	70.00	6)	70.00	7)	70.00	8)	70.00	9)	70.00
10)	70.00	11)	70.00	12)	70.00	13)	70.00	14)	70.00	15)	70.00	16)	70.00	17)	70.00	18)	70.00
19)	70.00	20)	70.00	21)	70.00	22)	70.00	23)	70.00	24)	70.00	25)	70.00	26)	70.00	27)	70.00
28)	70.00																
Y= 40																	
1)	70.00	2)	70.00	3)	70.00	4)	70.00	5)	70.00	6)	70.00	7)	70.00	8)	70.00	9)	70.00
10)	70.00	11)	70.00	12)	70.00	13)	70.00	14)	70.00	15)	70.00	16)	70.00	17)	70.00	18)	70.00
19)	70.00	20)	5.00	21)	6.00	22)	70.00	23)	70.00	24)	70.00	25)	70.00	26)	70.00	27)	70.00
28)	70.00																
Y= 39																	
1)	70.00	2)	70.00	3)	70.00	4)	70.00	5)	70.00	6)	70.00	7)	70.00	8)	70.00	9)	70.00
10)	70.00	11)	70.00	12)	70.00	13)	70.00	14)	70.00	15)	70.00	16)	70.00	17)	70.00	18)	70.00
19)	70.00	20)	5.00	21)	7.00	22)	12.00	23)	70.00	24)	70.00	25)	70.00	26)	70.00	27)	70.00
28)	70.00																
Y= 38																	
1)	70.00	2)	70.00	3)	70.00	4)	70.00	5)	70.00	6)	70.00	7)	70.00	8)	70.00	9)	70.00
10)	70.00	11)	70.00	12)	70.00	13)	70.00	14)	70.00	15)	18.00	16)	7.00	17)	5.00	18)	5.00
19)	7.00	20)	19.00	21)	16.00	22)	20.00	23)	25.00	24)	70.00	25)	70.00	26)	70.00	27)	70.00
28)	70.00																
Y= 37																	
1)	70.00	2)	70.00	3)	70.00	4)	70.00	5)	70.00	6)	70.00	7)	70.00	8)	70.00	9)	70.00
10)	50.00	11)	50.00	12)	45.00	13)	40.00	14)	34.00	15)	25.00	16)	20.00	17)	17.00	18)	16.00
19)	17.00	20)	20.00	21)	24.00	22)	28.00	23)	33.00	24)	70.00	25)	70.00	26)	70.00	27)	70.00
28)	70.00																
Y= 36																	
1)	70.00	2)	70.00	3)	70.00	4)	70.00	5)	70.00	6)	55.00	7)	56.00	8)	57.00	9)	58.00
10)	59.00	11)	58.00	12)	57.00	13)	46.00	14)	38.00	15)	30.00	16)	26.00	17)	25.00	18)	24.00
19)	25.00	20)	27.00	21)	31.00	22)	36.00	23)	39.00	24)	70.00	25)	70.00	26)	70.00	27)	70.00
28)	70.00																
Y= 35																	
1)	70.00	2)	70.00	3)	55.00	4)	70.00	5)	70.00	6)	59.00	7)	60.00	8)	63.00	9)	64.00
10)	65.00	11)	64.00	12)	61.00	13)	57.00	14)	44.00	15)	35.00	16)	32.00	17)	28.00	18)	30.00
19)	32.00	20)	35.00	21)	39.00	22)	42.00	23)	46.00	24)	70.00	25)	70.00	26)	70.00	27)	70.00
28)	70.00																
Y= 34																	

•• START HEAD SOLUTION - TIME : 18615.0 DAYS ••

ITERATION NO.	1	EPSILON =	.7366E+00	EMAX =	.2417E-02
ITERATION NO.	2	EPSILON =	.5536E+00	EMAX =	.1629E-02
ITERATION NO.	3	EPSILON =	.4810E+00	EMAX =	.1452E-02

•• TIME = 18615.00 NO. OF PARTICLES GENERATED = 106 TOTAL PARTICLES = 2933

EXXON-CRANDON PROJECT REPORT 9.2 EXAMPLE CASE

**** HEADS (FEET) AT TIME = 18615.0000 DAYS ****

NO. OF ITERATIONS = 3 EPSILON = .481 EMAX = .001

HEADS IN X DIRECTION (FROM 1500 FT. DATUM)

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
41	56.2	55.9	55.4	54.5	53.3	52.1	51.1	50.4	50.0	50.0	50.8	52.5	54.9	57.6	60.5	64.0	67.7	71.0	73.3	75.9	77.5	78.1	79.9	83.4	86.6
40	55.3	55.1	54.6	53.7	52.2	50.8	49.8	49.0	48.4	48.2	48.9	50.6	53.5	56.3	58.9	62.7	66.9	71.0	72.0	74.0	75.0	76.0	77.0	82.6	86.2
39	53.6	53.4	53.1	52.3	49.8	48.2	47.1	46.3	45.3	44.3	44.7	46.6	51.0	54.0	55.0	60.0	65.0	76.0	77.0	79.0	85.2	83.1	78.0	82.5	85.9
38	51.0	50.8	51.0	51.6	45.6	43.8	43.2	42.6	41.0	38.0	38.0	39.0	40.0	52.8	58.9	69.5	77.9	78.0	80.0	82.0	87.7	85.7	80.0	82.6	85.2
37	47.5	46.7	47.3	56.5	36.0	37.0	38.0	39.0	39.0	45.0	48.7	51.8	55.3	61.9	68.2	75.4	80.6	80.0	85.0	88.3	89.8	87.9	83.9	80.0	83.7
36	43.8	40.0	34.0	90.0	35.0	39.9	43.9	46.9	49.5	53.5	57.1	60.6	64.6	69.4	74.6	80.0	83.9	85.0	89.2	91.2	91.3	89.3	85.5	80.0	82.2
35	42.8	40.0	34.0	34.0	34.0	43.2	49.1	53.2	56.6	60.3	63.8	67.2	70.8	74.5	78.7	83.2	86.9	89.3	91.5	92.6	92.3	90.4	87.1	82.4	79.0
34	43.4	42.0	41.4	42.4	44.7	49.6	54.2	58.1	61.6	65.0	68.3	71.4	74.5	77.7	81.3	85.2	88.4	90.7	92.3	93.1	92.8	91.2	88.3	84.3	79.0
33	44.3	43.0	45.5	48.0	50.8	54.5	58.3	61.8	65.1	68.3	71.2	74.0	76.8	79.7	82.7	85.9	88.7	90.7	92.1	92.8	92.8	91.7	89.5	86.2	82.4
32	45.5	44.0	48.2	51.6	54.7	58.0	61.3	64.4	67.5	70.4	73.0	75.5	78.1	80.7	83.3	85.9	88.3	90.1	91.4	92.2	92.5	92.0	90.2	87.4	83.8
31	47.1	45.0	50.3	54.0	57.2	60.2	63.2	66.0	68.9	71.6	73.8	76.1	78.5	81.0	83.4	85.8	87.8	89.4	90.7	91.7	92.2	92.1	91.2	88.6	84.6
30	49.8	50.0	52.7	55.7	58.6	61.4	64.0	66.6	69.4	71.8	73.9	76.0	78.2	80.6	82.9	85.2	87.1	88.7	90.1	91.1	91.7	91.9	91.4	89.5	86.2
29	51.1	51.0	53.7	56.3	59.0	61.5	64.0	66.3	68.9	71.2	73.2	75.2	77.4	79.7	82.0	84.2	86.2	87.9	89.3	90.3	91.0	91.1	90.8	89.7	87.4
28	51.4	51.0	53.5	55.9	58.3	60.7	63.0	65.2	67.6	69.8	72.0	73.9	76.0	78.3	80.7	82.9	85.0	86.9	88.4	89.5	90.3	90.4	90.4	89.9	88.3

Y
DIREC

27 50.9 50.0 52.3 54.5 56.8 59.1 61.3 63.5 65.7 67.8 70.0 72.1 74.2 76.5 78.9 81.3 83.6 85.6 87.3 88.7 89.7 90.0 90.3 90.1 89.1

26 50.3 50.0 50.0 52.0 54.5 56.7 59.0 61.2 63.3 65.4 67.5 69.7 71.9 74.3 76.9 79.4 81.9 84.1 86.1 87.8 89.0 89.8 90.3 90.3 89.6

25 48.8 47.5 45.0 48.5 51.4 54.0 56.3 58.4 60.4 62.4 64.5 66.7 69.1 71.7 74.5 77.3 80.0 82.5 84.8 86.7 88.2 89.4 90.1 90.3 90.2

24 47.4 45.2 40.0 44.7 48.2 51.0 53.1 55.1 56.9 58.8 60.9 63.1 65.7 68.6 71.8 75.0 78.0 80.8 83.3 85.5 87.3 88.7 89.6 90.2 90.3

23 47.1 44.9 40.0 40.0 44.8 47.7 49.7 51.2 52.7 54.6 56.6 58.7 61.3 65.0 68.8 72.5 75.9 78.9 81.7 84.1 86.2 87.8 89.0 89.8 90.2

22 48.0 46.1 42.7 39.0 42.1 44.4 46.0 46.7 47.4 49.6 51.5 53.1 55.3 60.5 65.6 69.9 73.7 77.1 80.0 82.6 84.9 86.7 88.0 89.1 89.8

21 49.6 47.9 44.4 39.0 38.0 41.1 42.1 41.7 40.0 44.2 45.9 46.2 45.0 55.7 62.4 67.5 71.6 75.1 78.2 81.0 83.4 85.2 86.8 88.1 89.1

20 51.9 50.2 47.0 42.3 37.0 38.8 38.8 37.0 39.0 40.7 41.1 40.0 46.9 54.4 60.5 65.5 69.7 73.3 76.4 79.1 81.5 83.4 85.1 86.7 88.2

19 54.6 53.1 50.0 45.0 37.0 35.0 35.0 35.0 37.6 37.7 37.0 42.2 48.0 53.9 59.3 63.9 67.9 71.4 74.3 77.0 79.3 81.2 83.0 84.6 86.4

18 57.8 56.5 53.7 49.7 44.6 40.4 35.0 35.0 35.9 35.0 38.8 43.5 48.5 53.6 58.3 62.5 66.2 69.4 72.2 74.6 76.7 78.6 80.2 81.8 81.8

17 61.3 60.1 57.7 54.4 50.2 45.6 40.8 35.0 35.0 35.0 39.3 43.9 48.5 53.1 57.3 61.0 64.3 67.2 69.7 71.8 73.7 75.4 77.0 78.4 79.2

16 64.8 63.7 61.7 58.7 55.0 50.8 46.2 41.7 38.6 35.0 35.0 43.6 48.1 52.3 56.1 59.5 62.4 64.8 66.9 68.7 70.3 71.8 73.5 75.5 80.0

15 68.3 67.3 65.4 62.6 59.1 55.0 50.5 45.9 41.7 37.7 35.0 43.2 47.6 51.4 54.8 57.7 60.2 62.2 63.9 65.2 66.4 67.6 69.1 71.0 75.0

14 71.6 70.7 68.9 66.2 62.7 58.6 53.9 48.7 43.3 38.0 35.0 42.8 46.9 50.4 53.4 55.9 57.9 59.5 60.6 61.4 62.0 62.6 63.6 65.0 68.8

13 74.8 73.9 72.1 69.5 65.9 61.6 56.5 50.6 43.6 35.0 35.0 42.3 46.0 49.1 51.7 53.8 55.4 56.5 57.1 57.2 57.0 56.8 57.2 55.0 62.2

12 77.8 76.9 75.1 72.4 68.9 64.4 58.9 52.4 44.6 35.0 36.6 41.8 45.0 47.8 50.0 51.8 52.9 53.5 53.5 52.8 51.6 49.9 48.4 50.0 57.1

11 80.7 79.7 77.9 75.2 71.6 67.0 61.3 54.5 46.1 35.0 35.0 40.4 43.8 46.4 48.3 49.7 50.4 50.5 49.9 48.5 46.3 42.4 34.0 45.4 53.1

10 83.3 82.4 80.5 77.8 74.1 69.5 63.8 57.1 49.3 41.0 35.0 40.2 42.9 45.0 46.6 47.6 48.1 47.8 46.7 44.6 42.1 38.9 34.0 43.3 50.3

9 85.7 84.8 83.0 80.2 76.6 71.9 66.2 59.5 51.9 43.5 35.0 39.7 41.9 43.6 44.9 45.7 45.9 45.3 43.9 40.8 34.0 34.0 34.0 42.3 48.5

8 87.9 87.0 85.2 82.5 78.9 74.2 68.6 61.9 54.1 45.1 35.0 38.1 40.9 42.0 43.1 43.7 43.8 43.3 42.1 39.6 34.0 35.6 37.6 42.4 47.3

7 89.9 89.1 87.3 84.6 81.0 76.5 71.0 64.3 56.3 46.7 35.0 37.0 38.2 40.1 41.1 41.9 42.0 41.4 40.2 37.7 34.0 35.5 37.4 41.3 45.7
6 91.7 90.9 89.2 86.6 83.1 78.7 73.3 66.9 59.1 49.2 35.0 35.0 35.0 35.0 38.9 40.1 40.2 39.8 38.8 36.9 34.0 34.0 34.0 38.5 43.7
5 93.3 92.5 90.8 88.3 84.9 80.7 75.7 69.7 62.8 55.0 46.9 42.8 39.6 35.0 35.0 37.4 38.4 38.2 37.7 36.6 35.3 34.7 34.0 34.0 41.5
4 94.6 93.8 92.2 89.7 86.5 82.6 77.8 72.4 66.3 59.9 53.7 48.8 44.5 39.9 35.0 35.0 35.0 36.6 36.5 36.0 35.4 34.9 34.6 34.0 40.9
3 95.6 94.8 93.2 90.9 87.8 84.0 79.6 74.6 69.1 63.5 58.0 53.0 48.5 44.2 40.2 37.6 35.0 35.2 34.0 34.0 34.0 34.0 34.0 34.0 41.1
2 96.3 95.5 94.0 91.7 88.7 85.1 80.8 76.1 71.0 65.8 60.7 55.8 51.2 46.9 42.9 39.0 34.0 34.0 34.0 35.5 36.2 36.8 37.6 39.2 42.9
1 96.6 95.8 94.3 92.1 89.2 85.6 81.5 76.9 72.0 66.9 61.9 57.1 52.6 48.3 44.3 40.5 37.2 36.1 35.9 36.6 37.4 38.2 39.4 41.2 44.0

SLICE NUMBER 2 ITERATION # 3 TIME = 18615.000 DAYS

HEADS IN X DIRECTION (FROM 1500 FT. DATUM)

26 27 28

Y
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41 89.1 90.8 91.6

40 88.8 90.5 91.4

39 86.3 90.0 90.8

38 87.5 89.2 90.1

37 86.3 88.1 89.0

36 84.9 86.8 87.8

35 83.0 85.4 86.5

34 81.6 84.0 85.2

33 80.0 82.7 84.0

32 80.0 81.7 82.9

31 80.0 80.0 82.1

30 83.1 80.0 82.1

29 84.9 82.0 83.3

28 86.1 83.0 84.6

27 87.3 85.0 86.3

26 87.0 88.3

25 89.9 90.0 90.6

24 90.3 90.3 93.5

23 90.4 90.4 95.3

22 90.2 90.2 96.0

21 89.8 89.8 95.6

20 89.3 89.8 94.2

19 86.8 90.3 91.6

18 87.2 88.4 89.2

17 85.2 85.9 86.4

16 81.4 82.5 83.1

15 77.0 78.4 79.2

14 71.9 74.0 75.0

13 66.7 69.5 70.8

12 62.1 65.3 66.8

11 58.3 61.7 63.3

10 55.4 58.6 60.2

9 53.1 56.1 57.7

8 51.3 54.1 55.5

7 49.6 52.3 53.7

6 47.9 50.8 52.2

5 46.4 49.5 51.0

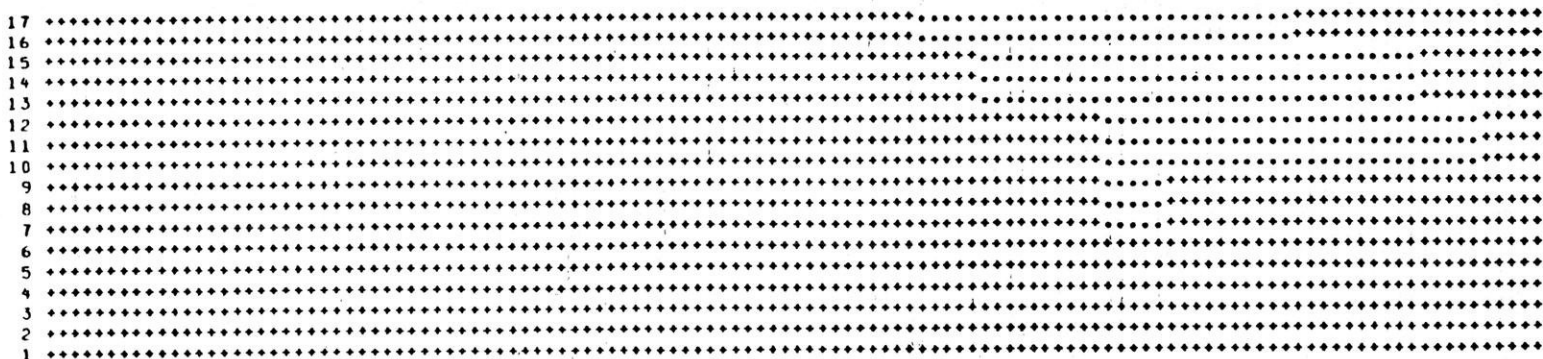
4 45.7 48.8 50.3

3 45.7 48.5 49.9

2 46.3 48.6 49.8

1 46.7 48.7 49.8

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*** SCALE ***
 * 0 1 2 3 4 5 6 7 8 9
 0.0 1.0 10. 20. 30. 40. 50. 60. 70. 80. 90. 100
 VALUES LESS THAN 0.0 DENOTED BY ?. VALUES GREATER THAN 100. DENOTED BY X

SLICE NUMBER 2 ITERATION # 3 TIME = 18615.000 DAYS

	X	
1	1	1
2	3	4

Y 12345678901234567890

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1 *****

*** SCALE ***
* 0 1 2 3 4 5 6 7 8 9
0.0 1.0 10. 20. 30. 40. 50. 60. 70. 80. 90. 100

VALUES LESS THAN 0.0 DENOTED BY ?. VALUES GREATER THAN 100. DENOTED BY X

APPENDIX C
Program Listing

```

1      PROGRAM RANWLK(INPUT,OUTPUT,TAPES=INPUT,TAPE6=OUTPUT,
2      1      DEBUG=OUTPUT,TAPE10,TAPE11,TAPE12,TAPE13)
3      C C$   ARRAYS
4      C C$   CALLS
5      C C$   TRACE
6      C
7      C.....
8      C
9      C   *** GOLDER ASSOCIATE'S GROUNDWATER COMPUTING SYSTEM ***
10     C
11     C           VERSION 6.1           04 DECEMBER 1981
12     C
13     C   ( COMBINED IADI AND PARTICLE PUSHER MODELS )
14     C
15     C   THIS VERSION IS A COMBINATION OF THE FINITE DIFFERENCE
16     C   HEAD SOLVER PROGRAM AND THE RANDOM WALK SOLUTE TRANSPORT
17     C   MODEL.
18     C
19     C   HEADS ARE COMPUTED BY SOLVING THE TRANSIENT EQUATION OF
20     C   GROUNDWATER PRESSURE DISTRIBUTION USING A FINITE
21     C   DIFFERENCE APPROXIMATION. THE GOVERNING EQUATION
22     C   AND SOLUTION METHODOLOGY ARE DOCUMENTED
23     C   IN:
24     C   "SELECTED DIGITAL COMPUTER TECHNIQUES FOR GROUND-
25     C   WATER RESOURCE EVALUATION"
26     C   T.A. PRICKETT AND C.G. LONNGUIST
27     C   ILLINOIS STATE WATER SURVEY BULLETIN 55
28     C   1971
29     C
30     C
31     C   THE RANDOM WALK MODEL TRACKS A MASS OF SOLUTE WHICH IS
32     C   IDEALIZED AS PARTICLES. TRANSPORT IS MODELED BY COMPUTING
33     C   THE X,Y COORDINATES OF EACH PARTICLE AS THEY ARE MOVED
34     C   BY BOTH GRADIENT INDUCED MOTION AND DISPERSIVE MOTION.
35     C
36     C   VERSION 6.0 INCLUDES THE FOLLOWING OPTIONS:
37     C
38     C   - SEPERATE HEAD AND PARTICLE GRID SPACING
39     C   - NODE BY NODE INPUT OF LEAKAGE PONDS
40     C   - VARIABLE THICKNESS ARRAY
41     C   - ONE LAYER MODEL (REMNANTS OF 2 LAYER MODEL REMAIN)
42     C   - RESTART FILES (INPUT AND OUTPUT)
43     C   - SINGLE CHARACTER OUTPUT MATRIX AT 1 IN. TO 2000 FT.
44     C   SCALE FOR 200 X 333.33 FT. PARTICLE GRICS
45     C
46     C.....
47     C
48     C
49     C
50     C   COMMON/PARMS/HEAD(28,41),HOLD(28,41),BOT(28,41),
51     C   1   T(28,41,2),R(28,41),MATL(28,41),
52     C   2   GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
53     C
54     C   COMMON/CONTRL/TSTEP,XX,NY,DX,DY,TIMMAX,OUTFR,NDATPR(10),LUNIT,
55     C   1   TUNIT,TITLE(P),TOL,MAXIT,NTYPE,NL,PARTVL,
56     C   2   IHEDIN,IHEDOT,ISELD
57     C

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58      COMMON/PARTIC/ X(25000),Y(25000),NOUT(150,150),DXPAR,DYPAR,
59      1 NXPAR,NYPAR,KELIM
60      C
61      COMMON/MATER/ NMAT,DL(20),DT(20),A(20),K(20),S(20),RE(20),POR(20)
62      C
63      COMMON/LEAK/ NLEAK,NNODES(20),NLPT(20),TM(20,20),RATE(20,20),
64      1      NODES(20,100,2)
65      C
66      COMMON/ERROR/ERRCOD,ERRARG(10)
67      C
68      REAL DX,K,TSTEP,ALPHA,TOL,ERRARG,TIMMAX
69      C
70      INTEGER NX,NY,NUMIT,TUNIT,LUNIT,OUTFRG,OUTKNT,ERRCOD
71      INTEGER MAXIT
72      INTEGER TITLE
73      C
74      C      GET DATE AND TIME
75      C
76      CALL CLOCK(ITIME)
77      CALL DATE(IDATE)
78      TIME = 0.0
79      C
80      C      READ IN DATA AND ECHO BACK TO PRINTER
81      C
82      ERRCOD = 0
83      CALL INPUT(ITIME,IDATE)
84      IF(ERRCOD.LE.0)GO TO 84
85      CALL ERPROC
86      IF(ERRCOD.GE.100) GO TO 900
87      84 CONTINUE
88      C
89      C
90      C      INITIALIZE VARIABLES
91      DTFAC = 1.
92      DTHAX = TSTEP
93      NTOTAL = 0
94      C
95      C      READ HEADS FROM RESTART FILE
96      C
97      IF(IHEDIN .LE. 0) GOTO 85
98      ERRCOD = 0
99      CALL HEDIN(TITLE,IHEDIN,NX,NY,HEAD,TIME,TUNIT,LUNIT,X,Y,
100      1      NTOTAL)
101      IF(ERRCOD .LE. 0) GOTO 85
102      CALL ERPROC
103      IF(ERRCOD .GE. 100) GOTO 900
104      85 CONTINUE
105      C
106      IF(ERRCOD .LE. 0) GOTO 90
107      CALL ERPROC
108      IF(ERRCOD .GE. 100) GOTO 900
109      90 CONTINUE
110      C      INITIALIZE ARRAYS
111      DO 100 I=1,NXPAR
112      DO 95 J=1,NYPAR
113      NOUT(I,J)=0
114      95 CONTINUE

```

```
115      100 CONTINUE
116          DO 110 I = 1,NX
117              DO 105 J=1,NY
118                  HOLD(I,J) = HEAD(I,J)
119      105 CONTINUE
120      110 CONTINUE
121      C***PLACE RESTART PARTICLES***
122      C
123          IF(IHEDIN.EQ.0) GO TO 111
124          DO 112 NP = 1,NTOTAL
125              CALL FIND(X(NP),Y(NP),DXPAR,DYPAR,NXPAR,NYPAR,NS,MS)
126              NOUT(NS,MS)=NOUT(NS,MS)+1
127      112 CONTINUE
128      111 CONTINUE
129      C      INITIALIZE COUNTER FOR OUTPUTTING HEADS EVERY "OUTFRQ" TIMESTEPS
130          OUTKNT = 1
131      C
132      C **** START TIME STEP LOOP ****
133      C
134          NT=0
135      120 CONTINUE
136          TIME = TIME + TSTEP
137          NT=NT+1
138          IF(MTYPE .EQ. 3) GO TO 140
139      C
140      C      SOLVE FOR HEADS IN UPPER LAYER
141      C
142          ERRCOD = 0
143          L = 1
144          CALL IADI(L,NT,TIME,NUMIT,E,EMAX)
145      C
146          IF(ERRCOD .LE. 0) GOTO 130
147              CALL ERPROC
148              IF(ERRCOD .GE. 100) GOTO 900
149      130 CONTINUE
150      140 CONTINUE
151          IF(MTYPE .EQ. 2) GOTO 150
152      C
153      C      SOLVE FOR LOWER LAYER HEADS
154      C
155          ERRCOD = 0
156          L = 2
157          CALL IADI(L,NT,TIME,NUMIT,E,EMAX)
158          IF(ERRCOD .LE. 0) GOTO 150
159              CALL ERPROC
160              IF(ERRCOD .GE. 100) GOTO 900
161      150 CONTINUE
162      C
163      C      COMPUTE INTER-LAYER LEAKAGE
164      C
165          IF(MTYPE .NE. 1) GOTO 160
166              ERRCOD = 0
167              CALL LEAK
168              IF(ERRCOD .LE. 0) GOTO 160
169                  CALL ERPROC
170                  IF(ERRCOD .GE. 100) GOTO 900
171      160 CONTINUE
```

```

172 C
173 C SOLVE FOR SOLUTE TRANSPORT IF PARTICLE VOLUME > 0
174 C
175 C IF(PARTVL.LE.0) GOTO 800
176 C
177 C GENERATE NEW PARTICLES
178 C
179 C ERRCOD=0
180 C CALL GENERAT(TIME,NTOTAL)
181 C IF(ERRCOD.LE.0) GO TO 162
182 C CALL ERPROC
183 C IF(ERRCOD.GE.100.) GO TO 900
184 C 162 CONTINUE
185 C
186 C SOLVE FOR SOLUTE TRANSPORT IF NO. OF PARTICLES > 0
187 C
188 C IF(NTOTAL.LE.0) GOTO 800
189 C
190 C COMPUTE NEW GRADIENTS
191 C
192 C EPRCOD = 0
193 C CALL GRADNT
194 C IF(ERRCOD.LE.0) GOTO 165
195 C CALL ERPROC
196 C IF(ERRCOD.GE.100) GOTO 900
197 C 165 CONTINUE
198 C
199 C DO 180 NP=1,NTOTAL
200 C
201 C FIND THE HEAD SOLUTION CELL THE PARTICLE IS IN
202 C
203 C CALL FIND(X(NP),Y(NP),DX,DY,NX,NY,NN,MM)
204 C MAT=MATL(NN,MM)
205 C
206 C IF POROSITY OF CELL IS ZERO, DO NOT MOVE PARTICLE
207 C
208 C IF(A(MAT).LE.0.0) GOTO 180
209 C
210 C FIND THE SOLUTE TRANS. CELL THAT PARTICLE IS IN
211 C
212 C CALL FIND(X(NP),Y(NP),DXPAR,DYPAR,NXP,HPAR,NS,MS)
213 C
214 C DECREMENT OUTPUT MATRIX PARTICLE COUNTER
215 C
216 C NOUT(NS,MS)=NOUT(NS,MS)-1
217 C
218 C MOVE PARTICLE
219 C WRITE(6,9993) NP,X(NP),Y(NP),MAT,DL(MAT),DT(MAT),A(MAT),
220 C I GX(NN,MM),GY(NN,MM)
221 C 9993 FORMAT(5X,"NP=",I5," X=",F8.2," Y=",F8.2," MAT=",I5,
222 C 1 " DL=",F8.2," DT=",F8.2," A=",F8.2," GX=",F10.5,
223 C 2 " GY=",F10.5)
224 C
225 C CALL MOVE(X(NP),Y(NP),DL(MAT),DT(MAT),A(MAT),HYP(NN,MM),
226 C 1 GX(NN,MM),GY(NN,MM),ISEED)
227 C
228 C FIND CELL THAT PARTICLE HAS MOVED TO

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```
229 C CALL FIND(X(NP),Y(NP),DXPAR,DYPAR,NXPAR,NYPAR,NS,MS)
230
231 C
232 C INCREMENT OUTPUT PARTICLE MATRIX COUNTER
233 C
234 NOUT(NS,MS)=NOUT(NS,MS)+1
235 180 CONTINUE
236 C
237 C OUTPUT RESULTS IF DESIRED
238 C
239 800 CONTINUE
240 IF(NT.NE.1.AND.NT.NE.OUTKNT) GOTO 810
241 OUTKNT = OUTKNT + OUTFRQ
242 C WRITE(6,9990) NDATPR
243 C9990 FORMAT(/5X,"I GOT INTO PRINT SECT.",10I1)
244 IFLAG = 0
245 IF(NDATPR(3).GE.1)CALL OUTPUT(TIME,NUMIT,E,EMAX)
246 IF(NDATPR(4).GE.1)CALL OUT2K(1,TIME,NUMIT,E,EMAX)
247 IF(NDATPR(5).GE.1.AND.NTOTAL.GT.0) CALL OUT2K(2,TIME,NUMIT,
248 1 E,EMAX)
249 IF(NDATPR(6).GE.1.AND.NTOTAL.GT.0)CALL PLOT3(NXPAR,NYPAR,
250 1 NOUT,TIME)
251 IF(NDATPR(7).GE.1.AND.NTOTAL.GT.0) IFLAG = 7
252 IF(IFLAG.NE.7) GOTO 807
253 WRITE(6,2010) TIME
254 2010 FORMAT(1H1,5X,"** PARTICLE COUNT AT TIME = ",F12.2,
255 1 " **//")
256 CALL PLOT
257 807 CONTINUE
258 C
259 IF(NDATPR(8).LE.0) GOTO 808
260 WRITE(6,2015) TIME
261 2015 FORMAT(1H1,5X,"** GRADIENTS AT TIME = ",F12.2,
262 1 " **//10X,"-- GX --//")
263 CALL PLOT3(GX,NX,NY)
264 WRITE(6,2020)
265 2020 FORMAT(/5X,"-- GY --//")
266 CALL PLOT3(GY,NX,NY)
267 WRITE(6,2025)
268 2025 FORMAT(/5X,"-- HYP --//")
269 CALL PLOT3(HYP,NX,NY)
270 808 CONTINUE
271 IF(NDATPR(10).GE.1)CALL OUT3K(TIME,NUMIT,E,EMAX)
272 C
273 C ELIMINATE INACTIVE PARTICLES AFTER EACH PRINTING
274 C
275 C IF(NTOTAL.GT.0) CALL ELIM(DX,DY,NX,NY,NTOTAL)
276 810 CONTINUE
277 IF(KELIM.EQ.1.AND.NTOTAL.GT.0)CALL ELIM(DX,DY,NX,NY,NTOTAL)
278 C WRITE(6,9991) NDATPR
279 C9991 FORMAT(/5X,"I GOT PAST PRINT SECTION ",10I1/)
280 C
281 C ADJUST TIME STEP
282 C
283 TSTEP = TSTEP * DTFAC
284 IF(TSTEP.GT. DTMAX) TSTEP = DTMAX
285 C
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PROGRAM RANWLK 74/175 OPT=0

FTN 5.0+508

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286 C **** END TIME STEP LOOP ****
287 C
288 C IF(TIME.LE.TIMMAX) GO TO 120
289 C
290 C CHECK FOR OUTPUTTING RESTART FILE
291 C
292 C IF(IHEDOT .LE. 0) GO TO 820
293 C ERRCOD = 0
294 C CALL HEDOUT(TITLE,IHEDOT,NX,NY,HEAD,TIME,TUNIT,LUNIT,X,Y,NTOTAL)
295 C IF(ERRCOD .LE. 0) GO TO 820
296 C CALL ERPROC
297 C IF(ERRCOD .GE. 100) GO TO 900
298 C 820 CONTINUE
299 C
300 C END RUN
301 C
302 C WRITE(6,2000)
303 C 2000 FORMAT(// " **** END PROGRAM ****" //)
304 C
305 C 900 STOP
306 C END
```



```

1      SUBROUTINE INPUT(ITIME,IDATE)
2      C CS   ARRAYS
3      C CS   TRACE
4      C CS   CALLS
5      C
6      C .....
7      C
8      C   FUNCTION - THIS SUBROUTINE READS IN THE REQUIRED DATA TO MODEL
9      C   A GIVEN PROBLEM. THE PARAMETERS ARE INPUT ACCORDING
10     C   TO FIXED FORMAT, ALL FIELDS ARE 10 CHARACTERS WIDE.
11     C   THE DATA IS ECHOED BACK TO THE PRINTER FILE AFTER
12     C   EACH LINE IS READ.
13     C
14     C   PARAMETERS-   SEE MANUAL
15     C
16     C .....
17     C
18     C   COMMON/PARMS/HEAD(28,41),HOLD(28,41),BOT(28,41),
19     C   1      T(28,41,2),R(28,41),MATL(28,41),
20     C   2      GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
21     C
22     C   COMMON/CONTRL/TSTEP,NX,NY,DX,DY,TIMMAX,OUTFRQ,NDATPR(10),LUNIT,
23     C   1      TUNIT,TITLE(8),TOL,MAXIT,MTYPE,NL,PARTVL,
24     C   2      IHEDIN,IHEDOT,ISEED
25     C
26     C   COMMON/PARTIC/ X(25000),Y(25000),NOUT(150,150),DXPAR,DYPAR,
27     C   1  NXPAR,NYPAR,KFLIM
28     C
29     C   COMMON/MATER/  AMAT,DL(20),DT(20),A(20),K(20),S(20),RE(20),POR(20)
30     C
31     C   COMMON/LEAK/  NLEAK,NNODES(20),NLPT(20),TM(20,20),RATE(20,20),
32     C   1      NODES(20,100,2)
33     C
34     C   COMMON/ERROR/ERRCOD,ERRARG(10)
35     C
36     C   REAL K,LAYER,PTEMP(50)
37     C
38     C   INTEGER ERRCOD,LUNIT,MAXIT,NX,NY,OUTFRQ,TITLE
39     C   INTEGER TUNIT,ITEMP(50)
40     C   CHARACTER*5 NANS(10),MTD(3)*25
41     C   DATA MTD/'LEAKY TWO LAYER','UNCONFINED ONE LAYER',
42     C   .      'CONFINED ONE LAYER'/
43     C
44     C   FORCE 1 LAYER MODEL
45     C
46     C   NL = 1
47     C
48     C   READ TITLE,UNITS AND SINGLE VALUE PARAMETERS
49     C
50     C   READ(5,1010) TITLE
51     C   1010 FORMAT(A10)
52     C   IF(EOF(5) .NE. 0)GOTO 900
53     C   WRITE(6,2010) ITIME,IDATE,TITLE
54     C   2010 FORMAT('1',/5X,B2('A')/5X,"",80X,""/5X,"",17X,
55     C   1"GOLDER ASSOCIATES GROUNDWATER COMPUTING SYSTEM",17X,""/
56     C   25X,"",80X,""/5X,"",25X,"EXXON MINERALS CRANDON PROJECT",25X
57     C   3,""/5X,"",80X,""/5X,"",23X,"GROUNDWATER IMPACT SCREENING MOD",

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115      WRITE(6,2200)
116      2200 FORMAT(/5X,"--- PRINTOUT OPTIONS ---",30X,"SELECTED?")
117      DO 130 NPROPT=1,10
118          NANS(NPROPT)=*NO *
119          IF (INDATPR(NPROPT).GE.1) NANS(NPROPT)=*YES*
120      130 CONTINUE
121      WRITE(6,2210) (NANS(NPROPT),NPROPT=1,10)
122      2210 FORMAT(/
123          *5X,*DUMP HEAD, BOTTOM AND MATERIAL TYPE ARRAY          *.6X,A/
124          *5X,*DUMP HEAD ARRAY ON PROGRAM GENERATED ABORT        *.6X,A/
125          *5X,*PLOT HEAD MATRIX, 20 COLUMNS PER PAGE             *.6X,A/
126          *5X,*PLOT HEAD MATRIX, 1 INCH TO 2000 FEET SCALE       *.6X,A/
127          *5X,*PLOT CONCENTRATION MATRIX, 1 INCH TO 2000 FEET SCALE*.6X,A/
128          *5X,*PLOT SYMBOLIC ACTIVE AND BOUNDARY PARTICLE SUMMARY *.6X,A/
129          *5X,*PRINT DETAILED PARTICLE COUNT BY CELL              *.6X,A/
130          *5X,*PRINT DETAILED GRADIENTS BY CELL                  *.6X,A/
131          *5X,*PRINT ITERATION INFORMATION FOR FINITE DIFFERENCE *.6X,A/
132          *5X,*PRINT SYMBOLIC CONCENTRATION CONTOURS (SUBGRID)    *.6X,A/)
133      C
134      C   READ DEFAULT VALUES
135      C
136      WRITE(6,2026)
137      2026 FORMAT(/5X,"--- DEFAULT VALUES ---")
138      READ(5,1027) HEAD(1,1),BOT(1,1),MATL(1,1),THICK(1,1)
139      1027 FORMAT(2F10.0,I10,F10.3)
140      IF (EOF(5).NE.0) GOTO 900
141      WRITE(6,2027) HEAD(1,1),LUNIT,BOT(1,1),LUNIT,MATL(1,1),
142          * THICK(1,1),LUNIT
143      2027 FORMAT(/
144          * 5X,*GROUNDWATER HEAD ELEVATION      =*,F10.0,1X,A4/
145          * 5X,*BASE OF AQUIFER ELEVATION      =*,F10.0,1X,A4/
146          * 5X,*MATERIAL TYPE                  =*,I10/
147          * 5X,*AQUIFER THICKNESS              =*,F10.3,1X,A4)
148      C
149      C   FILL IN DEFAULT VALUES IN ARRAYS
150      C
151      C
152      DO 15 I=1,NX
153          DO 10 J=1,NY
154              DO 8 L=1,NL
155                  HEAD(I,J)=HEAD(1,1)
156                  BOT(I,J)=BOT(1,1)
157                  MATL(I,J)=MATL(1,1)
158                  THICK(I,J)=THICK(1,1)
159      8   CONTINUE
160      10  CONTINUE
161      15  CONTINUE
162      C
163      C   READ INPUT BLOCKS
164      C
165      30 CONTINUE
166      READ(5,2033) DBLOCK
167      2033 FORMAT(A4)
168      IF (EOF(5).NE.0) GOTO 900
169      C   SEARCH FOR PROPER BLOCK
170      IF (DBLOCK .EQ. "HEAD") GOTO 45
171      IF (DBLOCK .EQ. "BOTT") GOTO 160

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172         IF(DBLOCK .EQ. "MATE") GOTO 500
173         IF(DPLOCK .EQ. "LEAK") GOTO 250
174         IF(DELOCK .EQ. "TYPE") GOTO 200
175         IF(DBLOCK .EQ. "THIC") GOTO 400
176         IF(DBLOCK .EQ. "END ") GOTO 800
177     C     UNKNOWN BLOCK NAME
178         ERRCOD = 104
179         GOTO 910
180     C
181     -----
182     C
183     C
184     READ INITIAL HEAD VALUES
185     C
186         45 CONTINUE
187         WRITE(6,2034)
188     2034 FORMAT(/5X,"--- INITIAL HEAD VALUES ---"/)
189     C
190         L=0
191         IF(MTYPE .NE. 1) GOTO 47
192         READ(5,1036) LAYER
193     1036 FORMAT(A5)
194         IF(LAYER .EQ. "UPPER") L=1
195         IF(LAYER .EQ. "LOWER") L=2
196         IF(L .NE. 0) GOTO 47
197     C     UNKNOWN LAYER
198         ERRCOD = 105
199         GOTO 910
200     47 CONTINUE
201         IF(L .EQ. 0) L=MTYPE-1
202         IF(MTYPE .EQ. 1) WRITE(6,2036) LAYER
203     2036 FORMAT(/5X,"--- ",A5," LAYER ---"/)
204     C
205         J=NY
206     50 CONTINUE
207         IF(J.LT.1) GO TO 60
208         READ(5,1035) (PTEMP(I),I=1,NX)
209     1035     FORMAT(HF10.0)
210         IF(EOF(5) .NE. 0) GOTO 900
211         DO 55 I=1,NX
212             IF(PTEMP(I) .NE. 0) HEAD(I,J)=PTEMP(I)
213         55     CONTINUE
214         WRITE(6,2035) (HEAD(I,J),I=1,NX)
215     2035     FORMAT(5X,15F8.2/)
216     C
217         J=J-1
218         GO TO 50
219     60 CONTINUE
220         GO TO 30
221     100 CONTINUE
222     C
223     READ BOTTOM VALUES
224     C
225     160 CONTINUE
226         WRITE(6,2060)
227     2060 FORMAT(/5X,"--- BOTTOM ELEVATIONS ---"/)
228     C

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229      L=0
230      IF(MTYPE .NE. 1) GOTO 162
231      READ(5,1036) LAYER
232      IF(LAYER .EQ. "UPPER") L = 1
233      IF(LAYER .EQ. "LOWER") L = 2
234      IF(L .NE. 0) GOTO 162
235      ERRCOD = 105
236      GOTO 910
237
162 CONTINUE
238      IF(L .EQ. 0) L = MTYPE -1
239      IF(MTYPE .EQ. 1) WRITE(6,2036) LAYER
240      J=NY
241
165 CONTINUE
242      IF(J.LT.1) GOTO 170
243      READ(5,1065) (PTMP(I),I=1,NX)
244      FORMAT(8F10.0)
1065      IF(EOF(5) .NE. 0) GOTO 900
245      DO 167 I=1,NX
246          IF(PTMP(I) .NE. 0) BOT(I,J)=PTMP(I)
247
167 CONTINUE
248      WRITE(6,2065) (BOT(I,J),II=1,NX)
249      FORMAT((5Y,15FR,2)/)
2065      J=J-1
250      GOTO 165
251
170 CONTINUE
252      GOTO 30
253
C
254      READ MATERIAL TYPE MATRIX
255
C
256
C
257
258
259
260      2075 FORMAT(//,5X,"--- MATERIAL TYPE MATRIX ---"//
261          133X,"CELL NUMBER IN X DIRECTION"//
262          21X,"CELL NUMBER",26X,"1",19X,"2"//
263          31X,"Y DIRECTION",7X,"( 1 2 3 4 5 6 7 8 9 0)",* 1 2 3 4 5 6 7 8*/
264          *1X,11(" "),8X,55(" "))
265
C
266      L=0
267      IF(MTYPE .NE. 1) GOTO 205
268      READ(5,1036) LAYER
269      IF(LAYER .EQ. "UPPER") L=1
270      IF(LAYER .EQ. "LOWER") L=2
271      IF(L .NE. 0) GOTO 205
272      ERRCOD = 105
273      GOTO 910
274
205 CONTINUE
275      IF(L .EQ. 0) L=MTYPE-1
276      IF(MTYPE .EQ. 1) WRITE(6,2036) LAYER
277      J=NY
278
210 CONTINUE
279      IF(J.LT.1) GOTO 220
280      READ(5,1080) (ITEMP(I),I=1,NX)
281      FORMAT(80I1)
1080      IF(EOF(5) .NE. 0) GOTO 900
282      DO 215 I=1,NX
283          IF(ITEMP(I) .NE. 0) MATL(I,J)=(ITEMP(I))
284
215 CONTINUE
285

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286          WRITE(6,2080) J,(MATL(II,J),II=1,NX)
287          FORMAT(4X,I2,14X,26I2)
288          J=J-1
289          GOTO 210
290          220 CONTINUE
291          GOTO 30
292
C
293          C READ LEAKAGE HISTORIFS
294          C
295          250 CONTINUE
296          WRITE(6,2085)
297          2085 FORMAT(/,5X,"--- LEAKAGE HISTORIES ---"/)
298          C
299          NLEAK = 1
300          C
301          C START LEAKAGE HISTORY LOOP
302          C
303          255 CONTINUE
304          READ(5,1090) NUM,NLPT(NLEAK),NNODES(NLEAK)
305          IF(EOF(5).NE.0) GOTO 900
306          1090 FORMAT(3I10)
307          IF(NUM.LE.0.) GOTO 270
308          NN = NNODES(NLEAK)
309          WRITE(6,2090) NUM,NLPT(NLEAK),NNODES(NLEAK)
310          2090 FORMAT(/,5X,"HISTORY NO.",I5,5X,"NO. OF LEAKAGE CURVE POINTS =",
311          1          I5,5X,"NO. OF NODES =",I5,/)
312          C
313          NLP=NLPT(NLEAK)
314          READ(5,1092)(TM(NLEAK,IR),RATE(NLEAK,IR),IR=1,NLP)
315          1092 FORMAT(2F10.0)
316          IF(EOF(5).NE.0)GO TO 900
317          C
318          WRITE(6,2092)TUNIT,LUNIT,TUNIT,(TM(NLEAK,IR),RATE(NLEAK,IR),
319          *          IR=1,NLP)
320          2092 FORMAT(16X,"LEAKAGE CURVE"/5X,34(,"-")/10X,"TIME",16X,"RATE"/
321          1          9X,*(,"A4,")/,10X,*(CU,,"A9,")/,10X,*(,"A4,")/
322          2          (5X,F10.2,11X,F10.2))
323          C
324          READ(5,1095) (NODES(NLEAK,I,1),NODES(NLEAK,I,2),I=1,NN)
325          1095 FORMAT(20I4)
326          IF(EOF(5).NE.0) GOTO 900
327          WRITE(6,2095)NUM,(NODES(NLEAK,I,1),NODES(NLEAK,I,2),I=1,NN)
328          2095 FORMAT(41X,"NODAL COORDINATES OF LEAKAGE FROM SOURCE",I5//
329          *          5X,12(" X",Y,"")//
330          *          (5X,12(I3,"",I3,3X)))
331          C CHECK FOR VALID LEAKAGE HISTORY
332          DO 260 NLK=1,NLP
333          IF(NLK.EQ.1)GO TO 260
334          NLK = NLK-1
335          IF(TM(NLEAK,NLK).GT.TM(NLEAK,NLK))GO TO 260
336          WRITE(6,2096)
337          2096 FORMAT(15X,"*FATAL ERROR LEAKAGE CURVE
338          1          TIME AXIS ERROR**"/)
339          C
340          260 CONTINUE
341          C
342          C

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343       NLEAK = NLEAK + 1
344       GOTD 255
345     C
346     C   END INPUT LOOP
347     C
348   270 CONTINUE
349       NLEAK = NLEAK - 1
350       GOTD 30
351     C
352     C   READ MATERIAL TYPE DEFINITION
353     C
354   300 CONTINUE
355       WRITE(6,2300) LUNIT, TUNIT, LUNIT, TUNIT, LUNIT, LUNIT
356   2300 FORMAT(/5X, "---- MATERIAL TYPE DEFINITION ----"/
357     * 77X, "LONGITUDINAL    TRANSVERSE"/
358     * 19X, "PERMEABILITY", 20X, "RECHARGE", 18X,
359     * "DISPERSIVITY    DISPERSIVITY"/
360     * 9X, "NUMBER    (", A4, ", ", A4, ")    STORATIVITY (CU.",
361     * A4, "/ ", A4, ")    POROSITY    (", A4, ")", 10X, "( ",
362     * A4, ")    RETARDATION"/)
363     C
364     NMAT = 1
365   310 CONTINUE
366       READ(5,1310) NUM,K(NMAT),S(NMAT),RE(NMAT),POR(NMAT),DL(NMAT),
367     * DT(NMAT),RT
368   1310 FORMAT(I10,7E10.3)
369       IF(EOF(5) .NE. 0) GOTD 900
370       IF(NUM .EQ. 0) GOTD 350
371     C
372       IF(RT.LE.0.0) RT=1.0
373     C
374       WRITE(6,2310) NUM,K(NMAT),S(NMAT),RE(NMAT),POR(NMAT),DL(NMAT),
375     * DT(NMAT),RT
376   2310 FORMAT(5X,I10,3(5X,E10.2),2(4X,E10.3),6X,E10.3,5X,E10.3)
377     C
378       A(NMAT) = 0.0
379       IF(POR(NMAT) .GT. 0) A(NMAT) = K(NMAT)*ISTEP/(POR(NMAT)*RT)
380     C
381       NMAT = NMAT + 1
382       GOTD 310
383     C
384   350 CONTINUE
385       NMAT = NMAT - 1
386       GOTD 30
387     C
388     C   READ IN THICKNESS ARRAY
389     C
390   400 CONTINUE
391       WRITE(6,2410) LUNIT
392   2410 FORMAT(/5X, "----AQUIFER THICKNESS (", A4, ")----")
393     C
394       J=NY
395   405 CONTINUE
396       IF(J.LE.0) GO TO 450
397       READ(5,1420) (PTMP(I), I=1,NX)
398   1420 FORMAT(8F10.0)
399       IF(EOF(5) .NE. 0) GO TO 900

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```
400      DO 410 I=1,NX
401          IF (PTEMP(I).GT.0.0) THICK(I,J)=PTEMP(I)
402      410 CONTINUE
403          WRITE(6,2430) J, (I, THICK(I,J), I=1, NX)
404      2430 FORMAT(1X, 'Y= ', I3 / (9(2X, I3, ' '), * , F8.2))
405      C
406          J=J-1
407          GO TO 405
408      450 CONTINUE
409          GO TO 30
410      C
411      C
412      C      COMPUTE TRANSMISSIVITIES
413      C
414      800 CONTINUE
415          IF (MTYPE .EQ. 2) GOTO 890
416          DO 810 I=1, NX
417              DO 810 J=1, NY
418                  MAT = MATL(I, J)
419                  IF (MTYPE .EQ. 1) T(I, J, 1) = K(MAT) * (HOT(I, J) - BOT(I, J))
420                  IF (MTYPE .EQ. 3) T(I, J, 1) = K(MAT) * THICK(I, J)
421                  T(I, J, 2) = T(I, J, 1)
422      810 CONTINUE
423      C
424      890 CONTINUE
425          WRITE(6, 2165)
426      2165 FORMAT(/, 5X, "***** INPUT COMPLETE *****")
427          GO TO 950
428      C
429      C      SET ERROR CODE
430      C
431      900 CONTINUE
432          ERRCOD = 102
433      910 CONTINUE
434          WRITE(6, 2070)
435      2070 FORMAT(/, 5X, "***** ERROR - INPUT INCOMPLETE *****")
436      C      SIGNAL INVALID MTYPE, MUST BE 1 LAYER ONLY IN THIS VERSION
437      920 CONTINUE
438          EKRCOD = 105
439          ERKARG(1) = 0
440      C
441      C      NORMAL EXIT
442      C
443      950 CONTINUE
444      C
445      C      CHECK FOR ARRAY DUMP
446          IF (NDATPR(1) .EQ. 0) GO TO 9000
447      C
448      C      DUMP ARRAYS (REVERSED FROM INPUT)
449      C
450          WRITE(6, 2176)
451      C      DUMP THE HEAD ARRAY
452          WRITE(6, 2171)
453          GO 955 L=1, NL
454          DO 955 J=1, NY
455              WRITE(6, 2174) (HEAD(I, J), I=1, NX)
456      955 CONTINUE
```


SUBROUTINE INPUT 74/175 OPT=0

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```
457 C DUMP BOTTOM ARRAY
458 WRITE(6,2177)
459 DO 958 L=1,NL
460 DO 958 J=1,NY
461 WRITE(6,2174) (BOT(I,J),I=1,NX)
462 958 CONTINUE
463 C DUMP THE "MATERIAL TYPE" ARRAY
464 WRITE(6,2172)
465 DO 960 L=1,NL
466 DO 960 J=1,NY
467 WRITE(6,2178) (MATL(I,J),I=1,NX)
468 960 CONTINUE
469 C
470 2171 FORMAT(/,5X,"HEADS",/)
471 2172 FORMAT(/,5X,"MATERIAL TYPE",/)
472 2173 FORMAT(/5X,"PERMEABILITY"/)
473 2174 FORMAT(5X,10F11.4)
474 2175 FORMAT(/5X,"STORAGE COEFFICIENTS"/)
475 2176 FORMAT(/5X,"ARRAY DUMP - UP-SIDE-DOWN FROM INPUT"/)
476 2177 FORMAT(/5X,"BOTTOM"/)
477 2178 FORMAT(5X,30I4)
478 9000 CONTINUE
479 C
480 RETURN
481 END
```

```

1      C
2      C
3      C      SUBROUTINE IADI(L,NT,TIME,NUMIT,E,EMAX)
4      C CS   ARRAYS
5      C CS   CALLS
6      C CS   TRACE
7      C
8      C .....
9      C
10     C      FUNCTION - THIS SUBROUTINE SOLVES FOR THE DISTRIBUTION OF HEADS
11     C      ACROSS THE FINITE DIFFERENCE GRID USING THE ITERATIVE
12     C      ALTERNATING DIRECTION IMPLICIT (IADI) SOLUTION TECH-
13     C      NIQUE. CORRECTION FOR TRANSMISSIVITY IN UNCONFINED
14     C      AQUIFER SIMULATION IS CONTROLLED BY THE PARAMETER "L":
15     C
16     C      L = 1 - UNCONFINED AQUIFER, CORRECT TRANSMISSIVITY
17     C      L = 2 - CONFINED AQUIFER, CONSTANT TRANSMISSIVITY
18     C
19     C .....
20     C
21     C      COMMON/PARMS/HLAD(28,41),HOLD(28,41),HOT(28,41),
22     1      T(28,41,2),R(28,41),MATL(28,41),
23     2      GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
24     C
25     C      COMMON/CONTRL/TSTEP,NX,NY,DX,DY,TIMMAX,CUTFRG,NDATPR(10),LUNIT,
26     1      TUNIT,TITLE(8),TOL,MAXIT,NTYPE,NL,PARTVL,
27     2      IHFCIN,IHEDOT,ISEED
28     C
29     C      COMMON/PARTIC/ X(25000),Y(25000),NOUT(150,150),DXPAR,DYPAR,
30     1      NXPAR,NYPAR,KELIM
31     C
32     C      COMMON/MATER/ NMAT,DL(20),DT(20),A(20),K(20),S(20),RE(20),POR(20)
33     C
34     C      COMMON/LEAK/ NLEAK,NNODES(20),NLPT(20),TM(20,20),RATE(20,20),
35     1      NODES(20,100,2)
36     C
37     C      COMMON/ERRCR/ERRCOD,ERRKARG(10)
38     C
39     C      REAL B(50),G(50),DLAST(28,41),K
40     C
41     C      INTEGER LRRCOD,TITLE,TUNIT,LUNIT
42     C      WRITE(6,10)L,NT,TIME
43     C 10  FORMAT(" * * * ARRIVED IN IADI * * * "/
44     1      "L =",I10,"NT=",I10,"TIME=",F10.2)
45     C      WRITE(6,9000)((T(I,J,2),I=1,NX),J=1,NY)
46     C9000  FORMAT(10F12.1)
47     C
48     C      IF(NDATPR(9).LE.0) GO TO 20
49     C      WRITE(6,2002) TIME,TUNIT
50     C 2002  FORMAT(1H1,75X," * * * START HEAD SOLUTION - TIME :",F9.1,
51     1      1X,A10," * * * //")
52     C 20  CONTINUE
53     C
54     C      LOAD RECHARGE ARRAY
55     C
56     C      DO 80 I=1,NX
57     C          DO 70 J=1,NY

```

```

58       MAT=MATL(I,J)
59       R(I,J) = RE(MAT)
60       70 CONTINUE
61       80 CONTINUE
62       C   WRITE(6,2003)
63       C2003 FORMAT(5X,"FLAG 1 "/)
64       C
65       IF(NLEAK.EQ.0)GO TO 87
66       DO 86 M=1,NLEAK
67         NLP=NLPT(M)
68         IF(TIME.LT.TM(M,1).OR.TIME.GT.TM(M,NLP)) GOTO 88
69         NNK = NNODES(M)
70         C   FIND LEAK RATE
71         C   WRITE(6,2004)
72         C2004 FORMAT(5X,"FLAG 2"/)
73         CALL INTERP(M,TIME,Q,TSTEP)
74         XRATIO = DX/DXPAR
75         YRATIO = DY/DYPAR
76         GRATIO = XRATIO*YRATIO
77         UNITQ = G/FLOAT(NNK)
78         DO 84 NN=1,NNK
79           IN = NODES(M,NN,1)
80           JN = NODES(M,NN,2)
81           XTEM=(IN-1)*DXPAR+DXPAR/2.
82           YTEM=(JN-1)*DYPAR+DYPAR/2.
83           CALL FIND(XTEM,YTEM,DX,DY,NX,NY,INI,JNI)
84           C   PRORATE RECHARGE ARRAY WITH POND LEAKAGES
85           MAT = MATL(INI,JNI)
86           R(INI,JNI) = R(INI,JNI)-(RE(MAT)/GRATIO)+UNITQ
87       CONTINUE
88       CONTINUE
89       86 CONTINUE
90       87 CONTINUE
91       C   PREDICT HEADS
92       C
93       DO 110 I=1,NX
94         DO 100 J=1,NY
95           D=HEAD(I,J)-HOLD(I,J)
96           HOLD(I,J)=HEAD(I,J)
97           F=1.0
98           IF(NT.LE.2) GOTO 90
99           IF(DLAST(I,J).EQ.0.0) GOTO 90
100          F=D/DLAST(I,J)
101          IF(F.GT.5.) F=5.0
102          IF(F.LT.0.0) F=0.0
103          90 CONTINUE
104          DLAST(I,J)=D
105          HEAD(I,J) = HEAD(I,J) *D*F
106          IF(HEAD(I,J).LE.BOT(I,J))HEAD(I,J)=BOT(I,J)+0.01
107        CONTINUE
108      100 CONTINUE
109      C   WRITE(6,2005)
110      C2005 FORMAT(5X,"FLAG3"/)
111      C
112      C   REFINE ESTIMATE OF HEADS BY IADI METHOD
113      C
114      NUMIT = 0

```

```

115      115 CONTINUE
116          E=0.0
117          EMAX=0.0
118          NUMIT=NUMIT + 1
119      C
120      C      EXTRA CMLCK FOR WATER LEVEL BELOW BOTTOM
121      C
122      C      DO 101 I=1,NX
123      C          DO 102 J=1,NY
124      C              XX=HEAD(I,J)-BOT(I,J)
125      C              YY=HEAD(I+1,J)-BOT(I+1,J)
126      C              IF(XX.LT.0.OR. YY.LT.0) WRITE(6,9999) I,J,XX,HEAD(I,J),
127      C          1          POT(I,J),YY,HEAD(I+1,J),BOT(I+1,J)
128      C9999          FORMAT(5X,2I10,6F10.3)
129      C 102 CONTINUE
130      C 101 CONTINUE
131      C
132      C      ADJUST TRANSMISSIVITIES IF AQUIFER IS UNCONFINED
133      C
134      C      IF(L.EQ.2) GOTO 135
135      C      DO 130 I=1,NX
136      C          DO 120 J=1,NY
137      C              MAT = MATL(I,J)
138      C              IF(I.LT.NX) T(I,J,2)=K(MAT) * SORT((HEAD(I,J)-
139      C          1          BOT(I,J))*(HEAD(I+1,J)-BOT(I+1,J)))
140      C              IF(J.LT.NY) T(I,J,1)=K(MAT)*SGRT((HEAD(I,J)-
141      C          1          POT(I,J))*(HEAD(I,J+1)-BOT(I,J+1)))
142      C              IF(L.EQ.'X') T(I,J,2) = (HEAD(I,J)-BOT(I,J)) * K(MAT)
143      C              IF(J.EQ.'Y') T(I,J,1) = (HEAD(I,J)-BOT(I,J)) * K(MAT)
144      C 120 CONTINUE
145      C 130 CONTINUE
146      C 135 CONTINUE
147      C
148      C      DO COLUMN CALCULATIONS
149      C
150      C      DXX=DX * DY
151      C      DO 250 II=1,NX
152      C          I=II
153      C      CHECK FOR DIRECTION ALTERATION
154      C      NTT=NT+NUMIT
155      C      IF(MOD(NTT,2).EQ.1) I=NX-I+1
156      C
157      C      CALCULATE B AND G ARRAYS
158      C
159      C      DO 220 J=1,NY
160      C          MAT = MATL(I,J)
161      C          BB=S(MAT)*DXX/TSTEP
162      C          DD=(HOLD(I,J)*S(MAT)+DXX/TSTEP)+R(I,J)
163      C          AA=0.0
164      C          CC=0.0
165      C          IF(J.EQ.1) GOTO 150
166      C              RR=-T(I,J-1)
167      C              RU=BB+T(I,J-1)
168      C          150 CONTINUE
169      C          IF(J.EQ.NY) GOTO 160
170      C              CC=-T(I,J,1)
171      C

```

```

172      BH=BB*T(I,J,1)
173      CONTINUE
174      IF(I.EQ.1) GOTO 170
175      BB=BB*T(I-1,J,2)
176      DD=DD+HEAD(I-1,J)*T(I-1,J,2)
177      CONTINUE
178      IF(I.EQ.NX) GOTO 180
179      BB=BB*T(I,J,2)
180      DD=DD+HEAD(I+1,J)*T(I,J,2)
181      CONTINUE
182      IF(J.GT.1)W=BB-AA*B(J-1)
183      IF(J.EQ.1)W=BB
184      B(J)=CC/W
185      IF(J.GT.1)G(J)=(DD-AA*G(J-1))/W
186      IF(J.EQ.1)G(J)=DD/W
187      CONTINUE
188      C
189      C RE-ESTIMATE HEADS
190      C
191      DH =ABS(HEAD(I,NY)-G(NY))
192      E=E+DH
193      IF(DH.GT.EMAX) EMAX=DH
194      HEAD(I,NY) = G(NY)
195      N = NY-1
196      CONTINUE
197      HA=G(N)-B(N)+HEAD(I,N+1)
198      DH = ABS(HA-HEAD(I,N))
199      E = E + DH
200      IF(DH.GT.EMAX) EMAX = DH
201      HEAD(I,N)=HA
202      N=N-1
203      IF(N.GT.0) GOTO 230
204      C
205      DO 240 N=1,NY
206      IF(HEAD(I,N).GT.BOT(I,N)) GOTO 240
207      E=E+BOT(I,N)+0.01 - HEAD(I,N)
208      HEAD(I,N)=BOT(I,N)+0.01
209      CONTINUE
210      C
211      CONTINUE
212      C
213      C READJUST TRANSMISSIVITY IF AQUIFER IS UNCONFINED
214      C
215      IF(L.EQ.2) GOTO 275
216      DO 270 J=1,NY
217      DO 260 I=1,NX
218      MAT = MATL(I,J)
219      IF(I.LT.NX) T(I,J,2)=K(MAT)*SQRT((HEAD(I,J)-BOT(I,J))
220      *(HEAD(I+1,J)-BOT(I+1,J)))
221      IF(J.LT.NY) T(I,J,1)=K(MAT)*SQRT((HEAD(I,J)-BOT(I,J))
222      *(HEAD(I,J+1)-BOT(I,J+1)))
223      IF(I.EQ.NX) T(I,J,2)=K(MAT)*(HEAD(I,J)-BOT(I,J))
224      IF(J.EQ.NY) T(I,J,1)=K(MAT)*(HEAD(I,J)-BOT(I,J))
260      CONTINUE
270      CONTINUE
275      CONTINUE
228      C

```

```

229      C   DO ROW CALCULATIONS
230      C
231      DO 400 JJ=1,NY
232      J=JJ
233      IF (MOD(JT+NUMIT,2).EQ.1) J=NY-J+1
234      DO 330 I=1,NX
235      MAT = MATL(I,J)
236      BB=S(MAT)*DXX/TSTEP
237      DD=(HOLD(I,J)*S(MAT)*DXX/TSTEP)*R(I,J)
238      AA=0.0
239      CC=0.0
240      IF (J.EQ.1) GOTO 280
241      BB=BB+T(I,J-1,1)
242      DD=DD+HEAD(I,J-1)*T(I,J-1,1)
243      280 CONTINUE
244      IF (J.EQ.NY) GOTO 290
245      DD=DD+HEAD(I,J+1)*T(I,J,1)
246      BB=BB+T(I,J,1)
247      290 CONTINUE
248      IF (I.EQ.1) GOTO 300
249      BB=BB+T(I-1,J,2)
250      AA=-T(I-1,J,2)
251      300 CONTINUE
252      IF (I.EQ.NX) GOTO 310
253      BB=BB+T(I,J,2)
254      CC=-T(I,J,2)
255      310 CONTINUE
256      IF (I.GT.1)W=BB-AA*B(I-1)
257      IF (I.EQ.1)W=BB
258      G(I)=CC/W
259      IF (I.EQ.1)G(I)=DD/W
260      IF (I.GT.1)G(I)=(DD-AA*G(I-1))/W
261      330 CONTINUE
262      C
263      C   RE-ESTIMATE HEADS
264      C
265      DH = APS(HEAD(NX,J)-G(NX))
266      E = E + DH
267      IF(DH .GT. E*MAX) EMAX = DH
268      HEAD(NX,J)=G(NX)
269      N=NX-1
270      340 CONTINUE
271      HA=G(N)-S(N)*HEAD(N+1,J)
272      DH =ABS(HEAD(N,J)-HA)
273      E = E + DH
274      IF(DH .GT. E*MAX) EMAX = DH
275      HEAD(N,J)=HA
276      N=N-1
277      IF (N.GT.0) GOTO 340
278      DO 350 N=1,NX
279      IF (HEAD(N,J) .GT. POT(N,J)) GOTO 350
280      E=E+DOT(N,J)+0.01-HEAD(N,J)
281      HEAD(N,J)=POT(N,J)+0.01
282      WRITE(6,2007) HEAD(N,J),POT(N,J),N,J
283      2007 FORMAT(//5X,"HEAD OF",F12.2," WENT BELOW BOTTOM",F12.2,
284      1      " AT NODE",215/)
285      350 CONTINUE

```

SUBROUTINE IADI

74/175 OPT=0

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286      C
287      C 400 CONTINUE
288      C
289      C PRINT ITERATION INFO
290      C
291      C IF(NCATPR(9).LE.0) GOTO 450
292      C WRITE(6,2010) NUMIT,E,FMAX
293      C 2010 FORMAT(5X,"ITERATION NO. ",15," EPSILON = ",E20.4," EMAX = ",E20.4)
294      C ERRCOD = 0
295      C IF(NUMIT.LT.MAXIT) GOTO 450
296      C ERRCOD = 101
297      C GOTO 901
298      C 450 CONTINUE
299      C
300      C CHECK FOR CONVERGENCE, PASS THRU AGAIN IF NOT CLOSED
301      C
302      C IF(E.GT.TOL) GOTO 115
303      C NORMAL EXIT
304      C 700 RETURN
305      C ITERATION LOOP ERROR
306      C 901 STOP
307      C END
```

```

1      C
2      C
3      C
4      SUBROUTINE LEAK
5      C C$   ARRAYS
6      C C$   CALLS
7      C C$   TRACE
8      C
9      C
10     C
11     C   FUNCTION - THIS SUBROUTINE COMPUTES THE INTERLAYER LEAKAGE BETWEEN
12     C   TWO AQUIFER LAYERS AS A FUNCTION OF THE DIFFERENCE
13     C   IN THE HEADS IN THE LAYERS, THE PERMEABILITY OF THE
14     C   UPPER LAER AND THE SATURATED DEPTH OF THE UPPER LAYER.
15     C
16     C
17     C   COMMON/PARMS/HEAD(28,41),HOLD(28,41),BOT(28,41),
18     C   1      T(28,41,2),R(28,41),MATL(28,41),
19     C   2      GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
20     C
21     C   COMMON/CONTRL/TSTEP,NX,NY,DX,DY,TIMMAX,OUTFRQ,NDATPR(10),LUNIT,
22     C   1      TUNIT,TITLE(8),TOL,MAXIT,MTYPE,NL,PARTVL,
23     C   2      IHEDIN,IHEDOT,ISEED
24     C
25     C   COMMON/PARTIC/ X(25000),Y(25000),NOUT(150,150),DXPAR,DYPAR,
26     C   1      NXPAN,NYPAN,KELIM
27     C
28     C   COMMON/MATER/ NMAT,DL(20),DT(20),A(20),K(20),S(20),RE(20),POR(20)
29     C
30     C   COMMON/LEAK/ NLEAK,NNODES(20),NLPT(20),TM(20,20),RATE(20,20),
31     C   1      NODES(20,100,2)
32     C
33     C   COMMON/ERROR/ERRCOD,ERRARG(10)
34     C
35     C   REAL K
36     C   INTEGER ERRCOD,OUTFRQ,TITLE,TUNIT
37     C   WRITE(6,10)
38     C 10  FORMAT(" * * * ARRIVED IN LEAK * * *")
39     C
40     C
41     C   OXX = DX * DY
42     C   DO 50 J=1,NY
43     C     DO 40 I=1,NX
44     C       DH = HEAD(I,J)-HEAD(I,J)
45     C       DLEN = HEAD(I,J)-BOT(I,J)
46     C       IF(DLEN .GT. 0.) GOTO 30
47     C       ERRCOD=103
48     C       GOTO 900
49     C   30  CONTINUE
50     C     MAT = MATL(I,J)
51     C     XLEAK = DXX * (DH/DLEN) * K(MAT)
52     C     R(I,J) = R(I,J) + XLEAK
53     C     R(I,J) = R(I,J) - XLEAK
54     C   40  CONTINUE
55     C   50  CONTINUE
56     C
57     C 900  CONTINUE
58     C   RETURN
59     C   END

```


1 SUBROUTINE OUTPUT(TIME,NUMIT,E,EMAX)

2 C C1 ARRAYS
3 C C1 CALLS
4 C C1 TRACE
5 C
6 C
7 C
8 C
9 C

10 C
11 C FUNCTION: PRINTS THE HEAD MATRIX VALUES IN BLOCK FORM.
12 C THE ORIGIN (NODE 1,1) IS PRINTED IN THE LOWER
13 C LEFT CORNER. PROBLEMS WITH MORE THAN 20 COLUMNS
14 C ARE PRINTED IN STRIPS, 20 COLUMNS PER PAGE.
15 C
16 C
17 C

18 C
19 C COMMON/PARML/HEAD(28,41),HOLD(28,41),GOT(28,41),
20 C 1 I(28,41,2),R(28,41),MATL(28,41),
21 C 2 CX(28,41),GY(28,41),HYP(28,41),THICK(28,41)

22 C
23 C COMMON/CONTROL/TSTEP,NX,NY,DX,DY,TIMMAX,OUTER,NDATPR(10),LUNIT,
24 C 1 TUNIT,TITLE(H),TOL,MAXIT,MTYPE,NL,PARTVL,
25 C 2 IMEDIN,IMEDOT,ISEED

26 C
27 C COMMON/PARTIC/ X(25000),Y(25000),NDUT(150,150),DXPAR,DYPAR,
28 C 1 NXPAR,NYPAR,KELIM

29 C
30 C COMMON/MATER/ NMAT,DL(20),DT(20),A(20),K(20),S(20),RE(20),PCR(20)

31 C
32 C COMMON/LEAK/ NLEAK,NNODES(20),NLPT(20),TY(20,20),RATE(20,20),
33 C 1 NDEF(20,100,2)

34 C
35 C COMMON/ERROR/ERRCOD,ERRARG(10)

36 C
37 C INTEGER TITLE,TUNIT,LUNIT,NX,NY,COLLP,COLPP,ERRCOD
38 C REAL K

39 C WRITE(6,10)TIME,NUMIT

40 C 10 FORMAT(" * * * ARRIVED IN OUTPUT * * *")

41 C 1 "TIME=",F10.2,"NUMIT=",I10)

42 C WRITE(6,2010) TITLE

43 C 2010 FORMAT("1",5X,H10)

44 C
45 C WRITE(6,2015) LUNIT,TIME,TUNIT,NUMIT,TSTEP

46 C 2015 FORMAT(/5X,"***** HEADS (" ,A4,") AT TIME = ",F10.4,1X,A4," *****"

47 C 1 //5X,"NUMBER OF ITERATIONS =",I5,

48 C 2 " TSTEP =",F10.4//)

49 C
50 C COMPUTE NUMBER OF SLICES READ TO PRINT MATRIX

51 C
52 C LA = 1

53 C IF(MTYPE .EQ. 3) LA = 2

54 C LAYER = " "

55 C 105 CONTINUE

56 C IF(MTYPE .EQ. 1 .AND. LA .EQ. 1) LAYER = "UPPER"

57 C IF(MTYPE .EQ. 1 .AND. LA .EQ. 2) LAYER = "LOWER"

IF(LAYER .NE. " ") WRITE(6,2017) LAYER

```
100 2017 FORMAT(/10X,"*** ",A5," LAYER ***"/)
101 C
102     COLPP = 20
103     NSLICE = 1
104 110 CONTINUE
105     NCOLS = COLPP * NSLICE
106     IF(NX .LE. NCOLS) GOTO 120
107     NSLICE = NSLICE * 1
108     GOTO 110
109 120 CONTINUE
110 C
111 C     COMPUTE NUMBER OF COLUMNS IN LAST SLICE
112 C
113     COLLP = NX - (COLPP*(NSLICE - 1))
114 C
115 C     PRINT MATRIX IN SLICES OF "COLPP" COLUMNS PER PAGE
116 C
117     NPAGE = 0
118     DO 200 NSL = 1, NSLICE
119         NPAGE = NPAGE + 1
120         IF(NSL .GT. 1) WRITE(6,2020) NSL, NCOLS, TIME, TUNIT
121         FORMAT("1",/5X," SLICE NUMBER ",I5,"X", "ITERATION #",
122             /1X,5X,"TIME = ",F10.5,1X,A10/////))
123         KI = ((NSL - 1) * COLPP) + 1
124         L = NSL * COLPP
125         IF(NPAGE .EQ. NSLICE) L = NX
126 C
127         J=NY
128         140 CONTINUE
129         IF(J.LT.1) GO TO 150
130         WRITE(6,2025) (IRAD(I,J),I=KI,L)
131         2025 FORMAT(5X,20F6.1)
132         J=J-1
133         GO TO 140
134     150 CONTINUE
135 C
136     200 CONTINUE
137     WRITE(6,2030)
138     2030 FORMAT("1")
139     IF(LA .EQ. 2) GOTO 220
140     IF(LTYPE .NE. 1) GOTO 200
141     LA = LA + 1
142     GOTO 155
143 400 CONTINUE
144 C
145     RETURN
146     END
```

```

1      SUPROUTINE OUT2K(CITYPE,TIME,NUMIT,E,EMAX)
2      C C$  ARRAYS
3      C C$  CALLS
4      C C$  TRACE
5
6      C .....
7      C
8      C
9      C  FUNCTION: PRINTS THE HEAD OR CONCENTRATION MATRIX VALUES IN BLOCK
10     C          FORM. THE ORIGIN (NODE 1,1) IS PRINTED IN THE LOWER
11     C          LEFT CORNER. PROBLEMS WITH MORE THAN 25 COLUMNS
12     C          ARE PRINTED IN STRIPS, 25 COLUMNS PER PAGE.
13
14     C  ARGUMENTS:
15     C    ITYPE - SELECTS TYPE OF OUTPUT
16     C             1 = PRINTS HEADS
17     C             2 = PRINTS CONCENTRATIONS
18
19     C    TIME - TIME SINCE SIMULATION BEGAN
20
21     C    NUMIT - NUMBER OF ITERATIONS
22
23     C .....
24     C
25     C
26     C  COMMON/PARMS/HEAD(28,41),HOLD(28,41),POT(28,41),
27     C 1      T(28,41,2),K(28,41),MATL(28,41),
28     C 2      GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
29
30     C  COMMON/CONT/L/ISTEP,IX,XY,DX,DY,TIMEAX,OUTFRQ,NDATPR(10),LUNIT,
31     C 1      TUNIT,TITLE(8),TOL,MAXIT,MTYPE,NL,PARIVL,
32     C 2      INELIN,INEDOT,ISLID
33
34     C  COMMON/PARTIC/ X(25000),Y(25000),HOUT(150,100),DXPAR,DYPAR,
35     C 1  NAPAR,NYPAR,KELIM
36
37     C  COMMON/MATER/ SMAT,TL(20),DT(20),A(20),K(20),S(20),RC(20),POR(20)
38
39     C  COMMON/LLAK/ BLEAK,NOOCES(20),NLPT(20),IN(20,20),RATE(20,20),
40     C 1  NOGES(20,100,2)
41
42     C  COMMON/ERRDR/ERRCOD,FRRARG(10)
43
44     C  INTEGER TITLE,TUNIT,LUNIT,NX,NY,COLLP,COLPP,OUTFRQ
45
46     C  REAL HP(28),K
47
48     C  WRITE(6,999) ITYPE,TIME,NUMIT
49     C 9992 FORMAT(//5X,"I GOT INTO OUT2K ",110,F10.2,110/)
50     C  WRITE(6,2010) TITLE
51     C 2010 FORMAT("1"//5X,A10)
52
53     C  IF(ITYPE .NE. 1) GOTO 90
54     C  WRITE(6,2015) LUNIT,TIME,TUNIT,NUMIT,E,EMAX
55     C 2015 FORMAT(/10X,".... HEADS (" ,A4," ) AT TIME = ",F10.4,1X,A4,
56     C 1  " ...."//15X,"NO. OF ITERATIONS =",15," EPSILON = ",F8.3,
57     C 2  " EMAX =",F8.3//)

```

```

90 CONTINUE
C
  IF(MTYPE .NE. 2) GOTO 100
  WRITE(6,2016) TIME,TUNIT,NUMIT,E,EMAX
2016 FORMAT(/10X,"**** CONCENTRATIONS (AS PERCENT POND LEAKAGE)",
1 " AT TIME =",F10.4,1X,A4," ****",/11X,"NO. OF ITERATIONS =",
2 " ",15," EPSILON =",F8.3," EMAX =",F8.3//)
100 CONTINUE
C
  COMPUTE NUMBER OF SLICES REQD TO PRINT MATRIX
C
  LA = 1
  IF(MTYPE .EQ. 2) LA = 2
  LAYER = " "
105 CONTINUE
  IF(MTYPE .EQ. 1 .AND. LA .EQ. 1) LAYER = "UPPER"
  IF(MTYPE .EQ. 1 .AND. LA .EQ. 2) LAYER = "LOWER"
  IF(LAYER .NE. " ") WRITE(6,2017) LAYER
2017 FORMAT(/10X,"*** ",A5," LAYER ***"/)
C
  COLPP = 25
  NSLICE = 1
110 CONTINUE
  NCOLS = COLPP * NSLICE
  IF(NX .LL. NCOLS) GOTO 120
  NSLICE = NSLICE + 1
  GOTO 110
120 CONTINUE
C
  COMPUTE NUMBER OF COLUMNS IN LAST SLICE
C
  COLLP = NX - (COLPP*(NSLICE - 1))
C
  PRINT MATRIX IN SLICES OF "COLPP" COLUMNS PER PAGE
C
  NPAGE = 0
  WRITE(6,9990) NSLICE,COLPP,COLLP,NX
9990 FORMAT(5X,"STARTING SLICE LOOP = ",NSLICE,COLPP,COLLP,NX=",9119)
  DO 200 NSL = 1,NSLICE
    IF(NSL .LE. 2) GOTO 125
    ANSL = FLOAT(NSL)
    WRITE(6,9991) ANSL
9991 FORMAT(5X,"ERROR = NSL =",F20.5)
    GO TO 900
  125 CONTINUE
  NPAGE = NPAGE + 1
  IF(NSL .GT. 1) WRITE(6,2020) NSL,NUMIT,TIME,TUNIT
2020 FORMAT("1",/5X," SLICE NUMBER ",15,5X," ITERATION #",
1 " ",5X," TIME = ",F10.3,1X,A4//)
  KI = (NSL - 1) * COLPP + 1
  L = NSL * COLLP
  IF(NPAGE .EQ. NSLICE) L = NX
C
  WRITE(6,2027)
2027 FORMAT(45X,"HEADS IN X DIRECTION (FROM 1500 FT. DATUM)")
  IF(NPAGE.GT.1) GO TO 130
  WRITE(6,2022)

```

```

115      GO TO 152
116      150 WRITE(6,2023)
117      152 CONTINUE
118      2023 FORMAT(/4X,' 26 27 28')
119      2022 FORMAT(5X,' 1 2 3 4 5 6 7 8 ',
120      1      " 9 10 11 12 13 14 15 16 ",
121      2      " 17 18 19 20 21 22 23 24 ")
122      3      " 25")
123      WRITE(6,2022)
124      2029 FORMAT(4X,'Y'/2X,'DIRC'/2X,'-----')
125      J=NY
126      140 CONTINUE
127      IF(J.LT.1) GO TO 159
128      CFAC = (PARTVL*100.)/(DX*DY)
129      DO 145 I=K1,L
130          MAT = MATL(I,J)
131          IF(CITYPE.EQ.1) HPC(I) = HEAD(I,J)-1500.
132          IF(CITYPE.EQ.2) HPC(I) = (CFAC/TRICK(I,J)) * ROUTE(I,J)/PUR
133          (MAT)
134      145 CONTINUE
135          IF(CITYPE.EQ.1) WRITE(6,2024) J,ORPC(I),I=K1,L)
136          IF(CITYPE.EQ.2) WRITE(6,2025) J,ORPC(I),I=K1,L)
137      2024 FORMAT(/2X,'I',25F5.1)
138      2025 FORMAT(/2X,'I',25F5.2)
139      J=J-1
140      GO TO 140
141      150 CONTINUE
142      C
143      WRITE(6,2024) NSL
144      2024 FORMAT(5X,'END PRINT LOOP, NSL =',150)
145      200 CONTINUE
146      WRITE(6,2030)
147      2030 FORMAT("1")
148      IF(LA.EQ.2) GOTO 200
149      IF(MTYPE.NE.1) GOTO 900
150      LA = LA + 1
151      GOTO 155
152      900 CONTINUE
153      C
154      RETURN
155      END

```

```

1      SUBROUTINE GRADNT
2      C CS   ARRAYS
3      C CS   CALLS
4      C CS   TRACE
5      C
6      C .....
7      C
8      C
9      C   THIS SUBROUTINE COMPUTES THE X AND Y GRADIENTS AT
10     C   EACH NODE FROM THE HEAD DISTRIBUTION COMPUTED BY THE
11     C   FINITE DIFFERENCES GROUNDWATER SOLUTION
12     C
13     C .....
14     C
15     C   COMMON/PARMS/HEAD(28,41),HOLD(28,41),COT(28,41),
16     C   1      T(28,41,2),R(28,41),MATL(28,41),
17     C   2      GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
18     C
19     C   COMMON/CONTRL/ISTEP,IX,NY,DX,DY,TIMMAX,OUTFRQ,NDATPR(10),LUNIT,
20     C   1      TUNIT,TITLE(8),TOL,MAXIT,MTYPE,NL,PARTVL,
21     C   2      IFRIN,IMEDOT,ISEED
22     C
23     C   COMMON/LEAK/ SLEAK,NNOLES(20),NLPT(20),IP(20,20),RATE(20,20),
24     C   1      SIZES(20,100,1)
25     C
26     C   COMMON/ERRCR/ERRCR0,ERRRFG(10)
27     C
28     C   INTEGER TITLE,LUNIT,TUNIT,OUTFRQ
29     C   WRITE(6,10)
30     C 10  FORMAT(' * * * ARRIVED IN GRADNT * * *')
31     C
32     C   SET MATRICES TO ZERO
33     C
34     C   DO 50 I=1,NX
35     C     DO 40 J=1,NY
36     C       HYP(I,J)=0.0
37     C       GX(I,J)=0.0
38     C       GY(I,J)=0.0
39     C 40  CONTINUE
40     C
41     C 50  CONTINUE
42     C
43     C   COMPUTE GRADIENTS AT EDGES OF MATRIX
44     C
45     C   DO 60 I=1,NX
46     C     GY(I,1)=(HEAD(I,1)-HEAD(I,2))/DY
47     C     GY(I,NY)=(HEAD(I,NY-1)-HEAD(I,NY))/DY
48     C 60  CONTINUE
49     C
50     C   DO 70 J=1,NY
51     C     GX(1,J)=(HEAD(1,J)-HEAD(2,J))/DX
52     C     GX(NX,J)=(HEAD(NX-1,J)-HEAD(NX,J))/DX
53     C 70  CONTINUE
54     C
55     C   COMPUTE INTERIOR MATRIX GRADIENTS
56     C
57     C   NXI=NX-1
58     C   NYI=NY-1

```

SUBROUTINE GRADT 747175 OPT=0

FTN 5.0+50R

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65      DO 100 I=1,NX
66      DO 90 J=1,NY
67      GX(I,J)=(HEAD(I-1,J)-HEAD(I+1,J))/(2*DX)
68      GY(I,J)=(HEAD(I,J-1)-HEAD(I,J+1))/(2*DY)
69      90 CONTINUE
70      100 CONTINUE
71      C
72      C COMPUTE HYPOTENUS FOR LATER USE
73      C
74      DO 120 I=1,NX
75      DO 110 J=1,NY
76      HYP(I,J)=SQRT(GX(I,J)*GX(I,J)+GY(I,J)*GY(I,J))
77      110 CONTINUE
78      120 CONTINUE
79      RETURN
80      END
```

```

1      SUBROUTINE HEAD(TITLE, IHEDIN, NX, NY, HEAD, TIME, TUNIT, LUNIT, X, Y,
2      NTOTAL)
3      C CS   ARRAYS
4      C CS   CALLS
5      C CS   TRACE
6      C
7      C
8      C
9      C   FUNCTION - THIS ROUTINE LOADS THE HEAD ARRAY FROM A RESTART
10     C   FILE.
11     C
12     C   PARAMETERS -
13     C     TITLE - TITLE OF RUN WHICH CREATED THE RESTART FILE,
14     C             READ FOR INFORMATION PURPOSES ONLY.
15     C     IHEDIN - FILE NUMBER THAT RESTART FILE IS ON.
16     C     NX - NUMBER OF VALUES IN THE X DIRECTION
17     C     NY - NUMBER OF VALUES IN THE Y DIRECTION
18     C     HEAD - HEAD ARRAY TO BE LOADED
19     C
20     C
21     C
22     C   COMMON/ERROR/ERRCOD,ERRRAG(10)
23     C   REAL HEAD(20,41),X(20000),Y(20000)
24     C   INTEGER TITLE(8),ERRCOD,TUNIT,LUNIT,TITLE1(8)
25     C   WRITE(6,10)TITLE,IHEDIN,NX,NY
26     C 10  FORMAT(' * * * ARRIVED IN HEAD * * * /
27     C     1   "TITLE=",A10/"IHEDIN=",I5,"NX=",I10,
28     C     2   "NY=",I10)
29     C
30     C
31     C   READ(IHEDIN,1015) TITLE1
32     C 1015 FORMAT(A10)
33     C   IF(.EQ.(IHEDIN).NE.0) GOTO 900
34     C   READ(IHEDIN,1017) TIME,ITUNIT,ILUNIT,MAX,NY,NTOTAL
35     C 1017 FORMAT(F10.0,6X,A4,6X,A4,3I10)
36     C   CHECK FOR PARAMETER MATCH
37     C   IF (ITUNIT.EQ.TUNIT) GO TO 20
38     C   WRITE(6,2016)ITUNIT,TUNIT
39     C 2016 FORMAT(/,5X,"** ERROR: TIME UNIT MISMATCH:",
40     C     1   " TIME UNIT ON RESTART FILE=",A10,2X,
41     C     2   " TIME UNIT ON INPUT FILE=",A10," **/")
42     C   GO TO 920
43     C 20   CONTINUE
44     C
45     C   IF(ILUNIT.EQ.LUNIT) GO TO 25
46     C   WRITE(6,2017)ILUNIT,LUNIT
47     C 2017 FORMAT(/,5X,"** ERROR: LENGTH UNIT MISMATCH:",
48     C     1   " LENGTH UNIT ON RESTART FILE=",A10,2X,
49     C     2   " LENGTH UNIT ON INPUT FILE=",A10," **/")
50     C   GO TO 930
51     C 25   CONTINUE
52     C
53     C   IF((NX.EQ.NX.AND.NY.EQ.NY) GO TO 30
54     C   WRITE(6,2018)MAX,NY,NX,NY
55     C 2018 FORMAT(/,5X,"** ERROR-GRID SPACING MISMATCH : NX,NY ON RESTART
56     C     1   FILE=",
57     C     2   TIME,2X,"NX,NY ON INPUT FILE=",

```



```
13      3      215,2X,('*')
14      GO TO 940
15      *9 CONTINUE
16      C
17      C
18      C WRITE TIME & PARTICLE COUNT
19      C
20      *RITE(6,2015)TITLE1,TIME,TUNIT,NTOTAL,LCRIT,TIME,TUNIT
21      2015 FORMAT(5X,'SIMULATION INITIALIZED BY RESTART FILE : ',8A10//
22      * 5X,'RESTART CONDITIONS :'/
23      * 7X,'TIME TO BEGIN SIMULATION =',2X,F10.3,1X,A4/
24      * 7X,'NUMBER OF PARTICLES =',1P/
25      * 7X,'HEADS IN ',4X,' AT TIME ',F10.3,1X,A4,1X,('*')//
26      C
27      DO 100 J=1,NX
28      READ(IHEDIN,1020) (HEAD(I,J),I=1,NX)
29      IF (EOF(IHEDIN) .NE. 0) GOTO 910
30      1020 FORMAT(F10.2)
31      C
32      WRITE(6,2020)J,(I,HEAD(I,J),I=1,NX)
33      2020 FORMAT(1X,'Y=',15/('(',2X,15,')',F8.2)))
34      C
35      100 CONTINUE
36      C
37      C READ IN PARTICLE LOCATIONS
38      C
39      READ(IHEDIN,1025) (X(I),Y(I),I=1,NTOTAL)
40      1025 FORMAT(F10.2)
41      C
42      C INPUT COMPLETE
43      C
44      GOTO 999
45      900 CONTINUE
46      C SIGNAL EMPTY RESTART FILE
47      ERRCOD = 105
48      ERRARG(1) = 1
49      ERRARG(2) = IHEDIN
50      GOTO 999
51      C
52      910 CONTINUE
53      C SIGNAL REACHED END OF FILE BEFORE COMPLETED LOAD
54      ERRCOD = 106
55      ERRARG(1) = 2
56      ERRARG(2) = 1
57      ERRARG(3) = J
58      GOTO 999
59      920 CONTINUE
60      ERRCOD = 107
61      GO TO 999
62      930 CONTINUE
63      ERRCOD = 108
64      GO TO 999
65      940 CONTINUE
66      ERRCOD = 109
67      C
68      999 CONTINUE
69      RETURN
70      END
```

```
1      C
2      SUBROUTINE HEDOUT(TITLE, IHEDOT, NX, NY, HEAD, TIME, TUNIT, LUNIT, X, Y,
3      1      NTOTAL)
4      C C%   ARRAYS
5      C C%   CALLS
6      C C%   TRACE
7      C
8      C .....
9      C
10     C FUNCTION - THIS ROUTINE WRITES A RESTART FILE FROM THE HEAD ARRAY
11     C             AT THE END OF A RUN.
12     C
13     C PARAMETERS:
14     C
15     C     TITLE - TITLE OF RUN WHICH IS WRITING THE RESTART FILE, FOR
16     C             INFORMATION PURPOSES ONLY
17     C     IHEDOT - FILE NUMBER ON WHICH TO WRITE RESTART FILE.
18     C     NX - NUMBER OF VALUES IN THE X-DIRECTION.
19     C     NY - NUMBER OF VALUES IN THE Y-DIRECTION
20     C     HEAD - HEAD ARRAY TO BE WRITTEN TO THE RESTART FILE.
21     C
22     C .....
23     C
24     REAL HEAD(20,41), X(25000), Y(25000)
25     INTEGER TITLE(0)
26     WRITE(6,10)
27     C 10  FORMAT(" * * * ARRIVED IN HEDOUT * * *")
28     C
29     WRITE(6,2010) IHEDOT
30     2010 FORMAT(/75X," * * HEADS BEING WRITTEN TO FILE # ",I5," * * //")
31     C
32     WRITE(IHEDOT,2020) TITLE
33     2020 FORMAT(A10)
34     WRITE(IHEDOT,2025) TIME, TUNIT, LUNIT, NX, NY, NTOTAL
35     2025 FORMAT(I10.2,6X,A4,6X,A4,5I10)
36     DO 100 J=1,NY
37     WRITE(IHEDOT,2030) (HEAD(I,J), I=1,NX)
38     2030 FORMAT("F10.2)
39     C
40     100  CONTINUE
41     WRITE(IHEDOT,2035) (X(I), Y(I), I=1,NTOTAL)
42     2035 FORMAT("F10.2)
43     RETURN
44     END
```

```

1      C
2      SUBROUTINE INTERP(M,TIME,G,ISTEP)
3      C C% ARRAYS
4      C C% CALLS
5      C C% TRACE
6      C
7      C .....
8      C
9      C FUNCTION - THIS ROUTINE INTERPOLATES A LEAKAGE RATE AT A GIVEN
10     C TIME FROM THE TIME-RATE CURVE INPUT FOR EACH LEAKAGE
11     C HISTORY.
12     C
13     C PARAMETERS:
14     C
15     C M - IDENTIFICATION NUMBER OF THE LEAKAGE HISTORY
16     C TIME - TIME VALUE FOR WHICH A LEAKAGE RATE IS NEEDED
17     C G - LEAKAGE RATE WHICH CORRESPONDS TO "TIME"
18     C ISTEP - TIME STEP OF RUN
19     C
20     C .....
21     C
22     COMMON/LEAK/ NLEAK,NNODES(20),NLPT(20),TM(20,20),RATE(20,20),
23     1 NGDES(20,100+2)
24     C
25     C WRITE(6,10)
26     C 10 FORMAT(" * * * ARRIVED IN INTERP * * *")
27     G = 0.0
28     TIME = TIME + (ISTEP/2.0)
29     C WRITE(6,9998) TIME,ISTEP,NLPT(M),RATE(M,1),TM(M,1)
30     C 9998 FORMAT(5X,"TIME=",F10.2," ISTEP=",F10.2, " NLPT=",I10,
31     C 1 " RATE(M,1)=",F10.2," TM(M,1)=",F10.2)
32     C CHECK IF VALUE IS IN BOUNDS
33     IF(TIME .LT. TM(M,1)) GOTO 900
34     NLPT = NLPT(M)
35     IF(TIME .GT. TM(M,NLPT)) GOTO 900
36     C INTERPOLATE LEAKAGE RATE
37     DO 100 IR=1,NLPT
38     IF(TIME .GT. TM(M,IR)) GOTO 100
39     IF(TIME .EQ. TM(M,IR)) GOTO 110
40     JR = IR - 1
41     DELR = RATE(M,IR) - RATE(M,JR)
42     DELT = TM(M,IR) - TM(M,JR)
43     G = RATE(M,JR) + ((DELR/DELT)*(TIME-TM(M,JR)))
44     GOTO 120
45     100 CONTINUE
46     GO TO 900
47     110 CONTINUE
48     C TIME IS EQUAL TO A SPECIFIED VALUE ON CURVE
49     G = RATE(M,IR)
50     120 CONTINUE
51     C
52     900 CONTINUE
53     C WRITE(6,2010) M,TIME,G
54     C 2010 FORMAT(/5X,"M,TIME & G =",I10,2F10.2)
55     RETURN
56     END

```

```

1      C
2      C   SUBROUTINE (GENERAT,TIME,NTOTAL)
3      C   CS   ARRAYS
4      C   CS   CALLS
5      C   CS   TRACI
6      C
7      C .....
8      C
9      C FUNCTION - THIS ROUTINE GENERATES PARTICLES IN THE TIMESTEP IN
10     C QUESTION AND ADDS THEM TO THE LIST OF PARTICLES.
11     C
12     C PARAMETERS :
13     C
14     C   ISEED - SEED VALUE FOR THE RANDOM NUMBER GENERATOR, WHICH
15     C         IS USED TO DISTRIBUTE THE INITIAL PARTICLE LOCATION.
16     C   TIME - TIME OF THE SIMULATION.
17     C   NTOTAL - POINTER WHICH INDICATES TOTAL NUMBER OF PARTICLES
18     C           THAT ARE IN SYSTEM. UPDATED FOR GENERATED PARTICLES
19     C
20     C .....
21     C
22     C
23     C   COMMON/PARMS/HEAD(28,41),HGLD(28,41),BOT(28,41),
24     C 1      T(28,41,2),R(28,41),MATL(28,41),
25     C 2      GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
26     C
27     C   COMMON/CONTRL/XTSTEP,NX,NY,DX,DY,TIMMAX,OUTFRQ,NDATPR(10),LUNIT,
28     C 1      TUNIT,TITLECR,TOL,MAXIT,MTYPE,NL,PARTVL,
29     C 2      INEDIR,INENOT,ISEED
30     C
31     C   COMMON/PARTIC/ X(25000),Y(25000),NOUT(150,150),DXPAR,DYPAR,
32     C 1      NYPAR,NYPAR,XELIM
33     C
34     C   COMMON/MATER/ DMAT,DL(20),DT(20),AC(20),R(20),S(20),RE(20),POR(20)
35     C
36     C   COMMON/LEAK/ NLEAK,NNODES(20),NLPT(20),TM(20,20),RATE(20,20),
37     C 1      NPP(20,100,100,2)
38     C
39     C   COMMON/ERROR/LRECOD,FRRARC(10)
40     C
41     C   INTEGER TITLE,LUNIT,TUNIT,OUTFRQ
42     C   REAL U(3)
43     C   WRITE(6,10)
44     C 10  FORMAT(" * * * ARRIVED IN GENERAT * * *")
45     C
46     C   NIN = 0
47     C   CYCLE THRU LEAKAGE HISTORIES
48     C   DO 100 N = 1,NLEAK
49     C     NLP = NLPT(N)
50     C     IF (TIME.LT.TM(N,1) .OR. TIME.GT.TM(N,NLP)) GOTO 100
51     C     DETERMINE LEAK RATE
52     C     CALL INTERP(TIME,C,XTSTEP)
53     C     NPP = INT((C+XTSTEP/PARTVL)*0.5)
54     C     WRITE(6,999) NPP,PARTVL
55     C 9999  FORMAT(5X,"NPP=",I10," PARTVL=",F15.2)
56     C     IF (NPP .LE. 0) GOTO 100
57     C     NIN = NIN + NPP

```

```
54 C CYCLE THRU PARTICLES
55 DO 80 NP = 1,NPP
56 C SELECT NODE IN WHICH TO LEAK PARTICLE
57 NTOTAL = NTOTAL + 1
58 IF (NTOTAL .GT. 2000) GO TO 900
59 CALL RAN4(15880,16777213,0,3,0)
60 INODE = INT(1+(C(1)-NNODES(N)))
61 XX = (FLOAT(NODES(N,INODE,1))*DXPAR)-(1.5*DXPAR)
62 YY = (FLOAT(NODES(N,INODE,2))*DYPAR)-(1.5*DYPAR)
63 X(NTOTAL) = XX + (DXPAR*(C(2)-0.5))
64 Y(NTOTAL) = YY + (DYPAR*(C(3)-0.5))
65 C WRITE(6,200) TIME,NLEAK,NP,X(NTOTAL),Y(NTOTAL),INODE
66 C2005 FORMAT(5X,"TIME=",F10.2," NLEAK=",I5," PART. #",I5,
67 C 1 " X=",F10.2," Y=",F10.2," NODE #",I5)
68 CALL FINE(X(NTOTAL),Y(NTOTAL),DXPAR,DYPAR,NXPAR,NYPAR,NN,MM)
69 NOUT(NN,MY) = NOUT(NN,MY) + 1
70 80 CONTINUE
71 100 CONTINUE
72 WRITE(6,101) TIME,NIN,NTOTAL
73 C210 FORMAT(7X,"* TIME =",F10.2,5X,"NO. OF PARTICLES",
74 C 1 " GENERATED =",I8,5X,"TOTAL PARTICLES =",I8)
75 C
76 C NORMAL FINISH
77 C
78 C GO TO 990
79 C
80 C SIGNAL PARTICLE COUNT OVEFLOW
81 C
82 C
83 C
84 C 900 CONTINUE
85 C
86 ERRCOD = 107
87 ERRARG(1) = 2
88 ERRARG(2) = NTOTAL
89 ERRARG(3) = TIME
90 C
91 C 999 CONTINUE
92 RETURN
93 END
```

SUBROUTINE FIND

74/175 OPT=0

FTN 5.0+538

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```
1      C
2      C .....
3      C
4      C
5      C
6      C      SUBROUTINE FIND(X,Y,IX,OY,DX,NY,MM)
7      C CS      ARRAYS
8      C CS      CALLS
9      C CS      TRACE
10     C      THIS SUBROUTINE FINDS WHICH CELL THE PARTICLES IS IN.
11     C      IF IT IS OUTSIDE THE AREA IT IS MOVED BACK TO THE NEAREST
12     C      SIDE CELL.
13     C
14     C .....
15     C      WRITE(6,10)
16     C 10  FORMAT(' * * * ARRIVED IN FIND * * *')
17     C
18     C      NN=INT((X/DX)+1.0)
19     C      MM=INT((Y/DY)+1.0)
20     C      IF (NN.GT.1) NN=1
21     C      IF (NN.GT.NX) NN=NX
22     C      IF (MM.GT.1) MM=1
23     C      IF (MM.GT.NY) MM=NY
24     C      WRITE(6,9990) X,Y,NN,MM
25     C 9990 FORMAT(5X,' FIND: X,Y',2F12.0,5X,' NN,MM =',2I10)
26     C      RETURN
27     C      END
```

```

1      C
2      SUBROUTINE MOVE (X,Y,DL,DT,A,HYP,GX,GY,ISEED)
3      C CS   ARRAYS
4      C CS   CALLS
5      C CS   TRACE
6      C
7      C .....
8      C
9      C
10     C   THIS SUBROUTINE MOVES THE PARTICLES TO THEIR NEW
11     C   POSITION ACCORDING TO SOLUTE TRANSPORT THEORY.
12     C   THE THEORY USED MOVES THE PARTICLE A DETERMINISTIC DISTANCE
13     C   DOWN GRADIENT, THEN MOVES IT A RANDOM AMOUNT ABOUT THAT NEW
14     C   CENTROID POINT. THE DETERMINISTIC MOVEMENT IS GIVEN BY:
15     C
16     C           (HYP*CON.*TIMESTEP SIZE)
17     C   DC = (MAX. GRADIENT) * -----
18     C           (PROSITY*RETARDATION)
19     C
20     C   THE LONGITUDINAL MOVEMENT OF THE PARTICLE IS THEN GIVEN BY:
21     C
22     C   DLONG = DC * SQRT(2*DC*DLONGIT. DISPERSIVITY)*RAND. NORM #1
23     C
24     C   SIMILARLY, THE TRANSVERSE MOVEMENT IS GIVEN BY:
25     C
26     C   DTRAN = SQRT(2*DC*TRANSVERSE DISPERSIVITY)*RAND. NORM #2
27     C
28     C   THE PROGRAM THEN RESOLVES THE MOVEMENT VECTOR INTO X AND Y
29     C   COMPONENTS, AND ADDS THEM TO THE ORIGINAL VALUES TO GIVE
30     C   THE FINAL LOCATION OF THE PARTICLE.
31     C
32     C   **NOTE** THERE IS NO UNIVERSAL AGREEMENT IN THE METHOD USED
33     C   TO COMPUTE THE LONGITUDINAL AND TRANSVERSE
34     C   MOVEMENT OF PARTICLES.
35     C
36     C .....
37     C
38     C   DIMENSION U(2)
39     C   WRITE(6,5)
40     C 5   FORMAT(" * * * ARRIVED IN MOVE * * *")
41     C   XO=X
42     C   YO=Y
43     C   IF (HYP.EQ.0) GOTO 10
44     C   CALL RAND( ISEED, 16777213, 1, 2, U)
45     C   DC=HYP*A
46     C   DLONG=U(1)*SQRT(2*DL*DC)+DC
47     C   DTRAN=U(2)*SQRT(2*DT*DC)
48     C   X=X+(GX*(DLONG-GY*DTRAN))/HYP
49     C   Y=Y+(GY*DLONG+GX*DTRAN)/HYP
50     C 10 CONTINUE
51     C   WRITE(6,999) X,Y,U(1),U(2)
52     C 9990 FORMAT(1X,"X=",F10.2," Y=",F10.2," U1=",F8.2," U2=",F8.2)
53     C   RETURN
54     C   END

```

```

1      C
2      C
3      SUBROUTINE ELM(CX,CY,NX,NY,NTOTAL)
4      C C%   ARRAYS
5      C C%   CALLS
6      C C%   TRACE
7      C
8      C .....
9      C
10     C   THIS SUBROUTINE SETS ALL BOUNDARY AREAS TO ZERO AFTER EACH PRINT
11     C   CYCLE SO THAT BOUNDARIES WILL INDICATE PARTICLES LOST PER PRINT
12     C   STEP ON EACH PRINTING. IN ADDITION THE LIST OF PARTICLES IS
13     C   REDUCED TO ACTIVE PARTICLES BY ELIMINATING THOSE IN
14     C   BOUNDARY AREAS.
15     C
16     C .....
17     C
18     C   COMMON/FARMS/HEAD(28,41),HOLD(28,41),HOT(28,41),
19     C   1      T(28,41,2),R(28,41),MATL(28,41),
20     C   2      GX(28,41),GY(28,41),HYP(28,41),THICK(28,41)
21     C
22     C   COMMON/PARTIC/ X(25000),Y(25000),NOUT(150,150),DXPAR,DYPAR,
23     C   1 NXPAR,NYPAR,KELIM
24     C
25     C   COMMON/WATER/ NMAT,CL(20),DT(20),A(20),F(20),S(20),FE(20),POR(20)
26     C   WRITE(6,5)
27     C 5  FORMAT(" * * * ARRIVED IN ELM * * *")
28     C
29     C
30     C   NBDY=0
31     C   KK=0
32     C 10  CONTINUE
33     C   KK=KK+1
34     C   IF(KK.GT.NTOTAL)GO TO 20
35     C   FIND WHICH MATERIAL CELL PARTICLE IS IN
36     C   CALL FIND(X(KK),Y(KK),IX,DY,NX,DY,NN,MM)
37     C   MAT=MATL(NN,MM)
38     C   IF(A(MAT).GT.0.160)GO TO 10
39     C
40     C   SWAP LAST PARTICLE IN LIST WITH STALLED PARTICLE
41     C
42     C   FIND WHICH SOLUTE CELL THE PARTICLE IS IN
43     C   CALL FIND(X(KK),Y(KK),DXPAR,DYPAR,NXPAR,NYPAR,NN,MM)
44     C   X(KK)=X(NTOTAL)
45     C   Y(KK)=Y(NTOTAL)
46     C   NTOTAL=NTOTAL-1
47     C   NBDY=NBDY+1
48     C   KK=KK-1
49     C
50     C   DECREMENT BOUNDARY PARTICLE MATRIX
51     C
52     C   NOUT(NN,MM)=NOUT(NN,MM)-1
53     C   GO TO 10
54     C 20  CONTINUE
55     C   WRITE(6,2000)NBDY,NTOTAL
56     C 2000 FORMAT(///5X,"BOUNDARY PARTICLES ELIMINATED IN THIS STEP =",
57     C 1      15," TOTAL NOW =",15)
58     C   RETURN
59     C   END

```


SUBROUTINE PLOTX 74/175 OPT=0

FTN 5.C+508

81/12/07. 12.15.58

PAGE 1

```
1 SUBROUTINE PLOTX(XV, NX, NY)
2 C 01 ARRAYS
3 C 01 CALLS
4 C 01 TRACE
5 DIMENSION XV(28,41)
6 C
7 C
8 C THIS IS A QUICKIE PLOT ROUTINE WHICH PRINTS THE MATRIX
9 C
10 DO 10 K=1, NY
11 J=NY-K+1
12 WRITE(6,2002)J
13 2002 FORMAT(1H, 'NY ROW = ', J)
14 WRITE(6,2000)(XV(I,J), I=1, NX)
15 2000 FORMAT(1H, '20F6.4)
16 10 CONTINUE
17 RETURN
18 END
```



```

1      C
2      C
3      C
4      C
5      C
6      C
7      C
8      C
9      C
10     C
11     C
12     C
13     C
14     C
15     C
16     C
17     C
18     C
19     C
20     C
21     C
22     C
23     C
24     C
25     C
26     C
27     C
28     C
29     C
30     C
31     C
32     C
33     C
34     C
35     C
36     C
37     C
38     C
39     C
40     C
41     C
42     C
43     C
44     C
45     C
46     C
47     C
48     C
49     C
50     C
51     C
52     C
53     C
54     C
55     C
56     C
57     C

```

SUBROUTINE ERPROC

C C5 ARRAYS
C C5 CALLS
C C5 TRACE

.....

FUNCTION - THIS SUBROUTINE HANDLES ERROR PROCESSING. THE
ERROR CODES ARE PASSED THRU THE PARAMETER
ERRCOD. ERROR CODES HANGING FROM 1-99 ARE NON-
FATAL INFORMATIONAL ERRORS. ERROR CODES GREATER
THAN 100 ARE FATAL AND RESULT IN THE TERMINATION
OF THE PROGRAM EXECUTION.
IN ADDITION, PARAMETERS OF INTEREST ARE
ALSO PASSED FOR PRINTING VIA "ARG", WITH
- ARG =THE ARGUMENT ARRAY
- ARG(1) =NUMBER OF ARGUMENTS
- ARG(2-10)=REAL ARGUMENTS

.....

COMMON/PARMS/HEAD(2P,41),HOLD(2P,41),DOT(2P,41),
1 T(2P,41,2),F(2P,41),MATL(2P,41),
2 GX(2P,41),GY(2P,41),HYP(2P,41),THICK(2P,41)

COMMON/CONTROL/ISTEP,IX,XY,DX,DY,TIMMAX,CUTFRQ,NDATPR(10),LUNIT,
1 TUNIT,TITLE(8),TOL,MAXIT,NTYPE,NL,PARTVL,
2 IMESIG,INFDOT,ISEED

COMMON/PARTIC/ X(25000),Y(25000),NOUTE(50,1500),LXPAR,CYPAR,
1 NXPAR,NYPAR,KELIM

COMMON/MATER/ KMAT,PL(20),DT(20),A(20),E(20),S(20),RE(20),PCR(20)

COMMON/LEAK/ NLEAK,NRODES(20),NLPT(20),IN(20,20),RATE(20,20),
1 NOCES(20,100,2)

COMMON/ERROR/ERRCOD,ERRARG(10)

INTEGER ERRCOD,TITLE,TUNIT,CUTFRQ
REAL ERRARG,HEAD,K
WRITE(6,10)
C 10 FORMAT(' * * * ARRIVED IN ERPROC * * *')

ERRARG=ERRARG(1)+1
IF (ERRCOD .EQ. 100) WRITE(6,2100)
C 100 FORMAT(/5X,' * * * FATAL ERROR #100 - NO FLOW BOUNDARY MATCH',
1 ' * NODE IS OUTSIDE GRID * * *')

IF (ERRCOD .EQ. 101) WRITE(6,2101)
C 101 FORMAT(/5X,' * * * FATAL ERROR #101 - SOLUTION OF HEADS DID NOT ',
1 ' CONVERGE IN MAXIMUM ALLOWED NUMBER OF ITERATIONS * * *')

IF (ERRCOD .EQ. 102) WRITE(6,2102)

```
115 CALL FINL(XA,YY,XX,XY,XX,XY,HEAD,MHEAD)
116 MAT = MATL(OM,AC,MHEAD)
117 IF (PBR(MAT).LT.0.0) HP(KOUNT) = "*"
118 IF (PBR(MAT).LE.0.0) GO TO 144
119 CONC = CFAC * NOUT(IH,J)/ (PBR(MAT) * THICK(CHEAD,MHEAD))
120 IF (CONC.GT.100.0) CONC.LT.0) GO TO 141
121 IF (CONC.GT.0.0.AND.CONC.LT.1.0) HP(KOUNT) = "*"
122 IF (CONC.GE.1.0.AND.CONC.LT.10.0) HP(KOUNT) = "0"
123 IF (CONC.GE.10.0.AND.CONC.LT.20.0) HP(KOUNT) = "1"
124 IF (CONC.GE.20.0.AND.CONC.LT.50.0) HP(KOUNT) = "2"
125 IF (CONC.GE.40.0.AND.CONC.LT.40.0) HP(KOUNT) = "3"
126 IF (CONC.GE.40.0.AND.CONC.LT.50.0) HP(KOUNT) = "4"
127 IF (CONC.GE.50.0.AND.CONC.LT.60.0) HP(KOUNT) = "5"
128 IF (CONC.GE.60.0.AND.CONC.LT.70.0) HP(KOUNT) = "6"
129 IF (CONC.GE.70.0.AND.CONC.LT.80.0) HP(KOUNT) = "7"
130 IF (CONC.GE.80.0.AND.CONC.LT.90.0) HP(KOUNT) = "8"
131 IF (CONC.GE.90.0.AND.CONC.LT.100.0) HP(KOUNT) = "9"
132
133 141 CONTINUE
134 IF (CONC.LT.0.0) HP(KOUNT) = "2"
135 IF (CONC.GE.10.0) HP(KOUNT) = "X"
136 IF (CONC.EQ.0.0) HP(KOUNT) = "."
137
138 144 KOUNT = KOUNT + 1
139 145 CONTINUE
140 IF (NPAGE.EQ.NSLICE) COLPP = COLLP
141 WRITE(6,2025) J,OMP(I),II=1,COLPP)
142 2025 FORMAT(2X,12,1X,120(A1))
143 J = J - 1
144 GO TO 140
145
146 150 CONTINUE
147 WRITE(6,2026)
148 2026 FORMAT(/41X,".... SCALE .... /26X," * 0 1 2 3 4 5"
149 1 " 6 7 8 9" /26X,
150 2 "0.0 1.0 10. 20. 30. 40. 50. 60. 70. 80. 90. 100"
151 3 //15X,"VALUES LESS THAN 0.0 (NOTED BY ?). VALUES GREATER THAN",
152 4 " 100. DENOTED BY *")
153
154 C
155 WRITE(6,9994) NSL
156 9994 FORMAT(5X,"END PRINT LOOP, NSL =",IAD)
157
158 200 CONTINUE
159 WRITE(6,2030)
160 2030 FORMAT("1")
161 IF (LA.EQ.2) GO TO 900
162 IF (MTYPE.NE.1) GO TO 900
163 LA = LA + 1
164 GO TO 105
165
166 900 CONTINUE
167
168 C
169 RETURN
170 END
```

```
2102 FORMAT(/5X,"** FATAL ERROR #102 - NOT ENOUGH INPUT DATA **"/)
C
  IF(ERRCOD.EQ.103) WRITE(6,2103)(ERRARG(I),I=2,NARG)
2103 FORMAT(/5X,"** FATAL ERROR #103 - PROBLEM DEFINITION RESULTS",
1 " IN NEGATIVE HEAD VALUES & RESTART FILE CREATED **",
2 /5X,"HEAD = ",F10.2,5X,"I,J = ",2F4.0)
C
  IF(ERRCOD.EQ.104) WRITE(6,2104)
2104 FORMAT(/5X,"** FATAL ERROR #104 - UNRECOGNIZABLE DATA BLOCK",
1 " NAME IN INPUT **"/)
C
  IF(ERRCOD.EQ.105) WRITE(6,2105)(ERRARG(I),I=2,NARG)
2105 FORMAT(/5X,"** FATAL ERROR #105 - RESTART FILE ON UNIT
1 NO.",I," IS EMPTY **"/)
C
  IF(ERRCOD.EQ.106) WRITE(6,2106)(ERRARG(I),I=2,NARG)
2106 FORMAT(/5X,"** FATAL ERROR #106 - RESTART FILE HAS
1 INSUFFICIENT DATA - I=",I4," J=",I4," **"/)
C
  IF(ERRCOD.EQ.107) WRITE(6,2107)(ERRARG(I),I=2,NARG)
2107 FORMAT(/5X,"** FATAL ERROR #107 - TOO MANY PARTICLES,
1 "TOTAL =",I4," A TIME =",F10.2,"RESTART FILE CREATED", " **"/)
C
  CHECK FOR FATAL ERROR
C
  IF(ERRCOD.GE.100) WRITE(6,2100)
2100 FORMAT(/5X,"** PROGRAM ABORTED **"/)
C
  CHECK FOR ARRAY DUMP OPTION
  IF(INJATPR(2).EQ.0) GOTO 200
  ARRAY DUMP WILL PUT NX VALUES OF X ON 1 LINE
  FOR NY LINES OF Y
  WITH A MAXIMUM NAXDP (NO MAX. NY)
C
  WRITE(6,1000)
C
  DO 160 I=1,NL
  DO 160 J=1,NY
  WRITE(6,1001) (HEAD(I,J),I=1,NX)
160 CONTINUE
1000 FORMAT(/5X,"HEAD ARRAY DUMP - UP-SIDE-DOWN"/)
1001 FORMAT(5X,20F6.1)
100 CONTINUE
  RETURN
  END
```

```

88 IF(MTYPE .EQ. 5) LA = 2
89 LAYER = " "
90
91 105 CONTINUE
92 IF(MTYPE .EQ. 1) JANI, IA .EQ. 1) LAYER = "UPPER"
93 IF(MTYPE .EQ. 1) JANI, LA .EQ. 2) LAYER = "LOWER"
94 IF(LAYER .NE. " ") WRITE(6,2017) LAYER
95 2017 FORMAT(10X,"*** ",A," LAYER ***")
96
97 C
98 COLPP = 120
99 NSLICE = 1
100
101 110 CONTINUE
102 NCOLS = COLPP * NSLICE
103 IF(NXPAR .LT. NCOLS) GO TO 120
104 NSLICE = NSLICE * 1
105 GO TO 110
106
107 110 CONTINUE
108
109 C COMPUTE NUMBER OF COLUMNS IN LAST SLICE
110 C
111 COLLP = NXPAR - (COLPP*(NSLICE - 1))
112
113 C PRINT MATRIX IN SLICES OF "COLPP" COLUMNS PER PAGE
114 C
115 NPAGE = 0
116 C WRITE(6,9990) NSLICE,COLPP,COLLP,NXPAR
117 9990 FORMAT(5X,"STARTING SLICE LOOP = NSLICE,COLPP,COLLP,NXPAR=",4I10)
118 DO 200 NSL = 1,NSLICE
119 IF(NSL .LE. 2) GO TO 125
120 XNSL = FLOAT(NSL)
121 WRITE(6,9991) XNSL
122 9991 FORMAT(5X,"ERROR = NSL =",E20.5)
123 GO TO 200
124 CONTINUE
125 NPAGE = NPAGE + 1
126 IF(NSL .GT. 1) WRITE(6,2020) NSL,NUMIT,TIME,TUNIT
127 2020 FORMAT("1",/5X," SLICE NUMBER ",15,5X,"ITERATION #",
128 1 /5X,"TIME = ",F10.3,1X,4977777)
129 IF(NSL.EQ.1)WRITE(6,2024)
130 IF(NSL.EQ.2)WRITE(6,2023)
131 2023 FORMAT(15X,"X"/6X,"1 1 1"
132 * 6X,"2"/6X,"3"/6X,"4"/
133 * 3X,"Y"/2X,"2"/(1234567890))
134 2024 FORMAT(63X,"X"/105X,"1 1 1" /6X,
135 1 9X,"1"/9X,"2"/9X,"3"/9X,"4"/9X,"5"/9X,"6"/9X,"7"/9X,"8"/9X,"9"/9X
136 /,"0"/9X,"1"/9X,"2"/9X,"Y"/2X,"2"/(1234567890))
137
138 KI=((NSL-1)*COLPP) + 1
139 L = NSL * COLPP
140 IF(NPAGE .EQ. NSLICE) L = NXPAR
141
142 C
143 J=NXPAR
144 140 CONTINUE
145 KOUNT=1
146 IF(J.LT.1) GO TO 150
147 CFAC = (PARTVL*100.)/(UXPAR*OYPAR)
148 DO 145 IH=KI,L
149 XX=(IH-1)*OXPAP+UXPAR/2.
150 YY=(J-1)*OYPAR+OYPAR/2.

```



```
1 C
2 SUBROUTINE OUT3K(TIME,NUMIT,FMAX)
3 C
4 C C1 ARRAYS
5 C C1 CALLS
6 C C1 TRACE
7 C
8 C
9 C
10 C FUNCTION: PRINTS THE CONCENTRATION MATRIX VALUES IN BLOCK
11 C FORM. THE ORIGIN (NODE 1,1) IS PRINTED IN THE LOWER
12 C LEFT CORNER.
13 C
14 C ARGUMENTS:
15 C
16 C TIME - TIME SINCE SIMULATION BEGAN
17 C
18 C NUMIT - NUMBER OF ITERATIONS
19 C
20 C
21 C
22 C
23 COMMON/PARMS/HEAD(25,41),HOLE(28,41),SCT(28,41),
24 1 T(28,41,2),P(28,41),MATL(28,41),
25 2 CA(28,41),CY(28,41),HYP(28,41),THICK(28,41)
26 C
27 COMMON/CONTROL/TSTEP,NX,NY,DX,DY,TIMMAX,CUTFRQ,NDATPR(10),LUNIT,
28 1 TUNIT,TITLE(9),TOL,MAXIT,RTYPE,NL,PARTVL,
29 2 INEQIN,INEQOT,ISEED
30 C
31 COMMON/PARTIC/ X(25000),Y(25000),NOUT(150,150),DXPAR,DYPAR
32 1,NXPAR,NYPAR,KEFLIM
33 C
34 COMMON/MATE4/ NMAT,EL(20),DT(20),A(20),K(20),S(20),RE(20),POR(20)
35 C
36 COMMON/LEAK/ NLEAK,NNODES(20),NLPT(20),TM(20,20),RATE(20,20),
37 1 NNODES(2,100,2)
38 C
39 COMMON/ERROR/ERRCOD,ERRAPG(10)
40 C
41 INTEGER TITLE,TUNIT,LUNIT,NX,NY,COLLP,COLPP,OUTFRQ,HP(120)
42 C
43 C
44 WRITE(6,9992) ITYPE,TIME,NUMIT
45 C9992 FORMAT(//5X,"I GOT INTO OUT3K ",I10,F10.2,I10/)
46 WRITE(6,2010) TITLE
47 2010 FORMAT("1"//5X,#A10)
48 C
49 WRITE(6,2016) TIME,TUNIT,NUMIT,F,FMAX
50 2016 FORMAT(//10X,"**** CONCENTRATIONS (AS PERCENT POND LEAKAGE)",
51 1 " AT TIME =",F10.4,1X," ",****"//,1'X,"NO. OF ITERATIONS =",
52 2 15," EPSILON =",F8.3," LMAX =",F8.3//)
53 C
54 C COMPUTE NUMBER OF SLICES REQD TO PRINT MATRIX
55 C
56 LA = 1
57 MTYPE=3
```

75
74
73
72
71
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